TRANSPORTATION CABINET DEPARTMENT OF HIGHWAYS JEFFERSON COUNTY I-65 SOUTHBOUND OVER OHIO RIVER PLAN SET A – TRUSS BEARING REPAIR

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			SHEET	INDEX	
	SI	TITI	_e sheet		
	S2-S3	GEN	ERAL NOTES		
	S4	BRI	DGE PLAN AND ELEVAT	ION	
	S5	PIEF	R IL PLAN		
	<u>\$6</u>	PIEF	₹ 6L PLAN		
	<u> </u>	PIEF	R IL TEMP. JACKING S	UPPORI	
	58	PIEF	CL DE CEN DE DENO		
	23		CL DS GEN. Z REMU		
	510	TEM	TOL US GEN. J REIRU DADADA HAID DAWN D		
	511 512	TEM	PORARY HOLD DOWN P	IER 6	
	513-514	HOL	D DOWN DETAILS		
	S15	PIEF	R IL BRG PIN REPLACE	MENT	
	S16	PIEF	R 6L US BRG PIN REP	LACEMENT	
	S17	PIEF	R 6L DS BRG PIN REP	LACEMENT	
	S18	PIEF	R 6L DS BRG CONCRET	E REPAIR	
	S19	WIN) LOCK REPAIR AT PI	ER 6L	
		FOR	INFORMATION ONLY S	HEETS	
		COU	NTERWEIGHT DETAILS	- JFK BRIDGE REHAB	ILITATION
		WIN	D TRANSFER DETAILS	- DESIGN DRAWING	
		L W I N I	J IRANSFER DETAILS	- SHOP DRAWING	
			<u>SPECIAL</u>	NOTES	
	FOR CO	NTRA(CT COMPLETION DATE	AND LIQUIDATED DAM	IAGES
	ON E	BRIDG	E REPAIR CONTRACT		
	FOR POP	RTABL	E CHANGEABLE MESSA	GE SIGNS	
	FOR TEN	MPOR/	ARY WORKSITE SPEED	LIMIT SIGN ASSEMBL	Y
	FOR TR	AFFIC	CONTROL ON BRIDGE	REPAIR CONTRACTS	
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BY: JCS DATE: DE

BY: MJB DATE: DE

SPECIFICATIONS

ALL REFERENCES TO THE STANDARD SPECIFICATIONS ARE TO THE CURRENT EDITION OF THE KENTUCKY DEPARTMENT OF HIGHWAYS STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, AND CURRENT SUPPLEMENTAL SPECIFICATIONS. ALL REFERENCES TO THE AASHTO SPECIFICATIONS ARE TO THE FOURTH EDITION 2017 AASHTO LRFD BRIDGE CONSTRUCTION SPECIFICATION AND 2020 AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 9TH EDITION.

COMPLETION OF THE STRUCTURE

THE CONTRACTOR IS REQUIRED TO COMPLETE THE STRUCTURE IN ACCORDANCE WITH THE PLANS AND SPECIFICATIONS. MATERIAL, LABOR OR CONSTRUCTION OPERATIONS, NOT OTHERWISE SPECIFIED, ARE TO BE INCLUDED IN THE BID ITEM MOST APPROPRIATE FOR THE WORK INVOLVED. THIS MAY INCLUDE REMOVAL OF ALL, OR PARTS, OF EXISTING STRUCTURES, PHASE CONSTRUCTION, INCIDENTAL MATERIALS, TEMPORARY WORKS, LABOR OR ANYTHING ELSE REQUIRED TO COMPLETE THE STRUCTURE.

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THE CONTRACTOR IS RESPONSIBLE FOR MAKNG A SITE INSPECTION TO BECOME FAMILIAR WITH THE WORK TO BE DONE AND TO MAKE APPROPRIATE ALLOWANCES FOR ALL WORK INCLUDED IN LUMP SUM BIDS. A SUITABLE METHOD OF PERFORMING THE WORK DESCRIBED HEREIN SHOULD BE INVESTIGATED. SUBMISSION OF A BID WILL BE CONSIDERED EVIDENCE OF THIS INVESTIGATION HAVING BEEN MADE. THE CONTRACTOR WILL NOT BE PAID EXTRA BECAUSE OF SITE CONDITIONS.

MAINTENANCE OF TRAFFIC

TRAFFIC SHALL BE MAINTAINED AT ALL TIMES IN ACCORDANCE WITH THE SPECIAL NOTE.

WELDING SPECIFICATIONS

ALL WELDING AND WELDING MATERIALS, EXCEPT FOR REINFORCEMENT, SHALL CONFORM TO JOINT SPECIFICATIONS ANSI/AASHTO/AWS DI.5 BRIDGE WELDING CODE 2015, 6TH EDITION. NON-DESTRUCTIVE TESTING BY THE CONTRACTOR WILL NOT BE REQUIRED. PAYMENT FOR WELDING, WELDING MATERIALS, STRAIGHTENING, ALTERING AND BURNING NEW OR EXISTING STEEL SHALL BE INCIDENTAL TO THE APPROPRIATE PAY ITEMS.

DIMENSIONS

DIMENSIONS SHOWN ON THE PLANS ARE TAKEN FROM THE ORIGINAL CONTRACT PLANS, SUBSEQUENT RECONSTRUCTION AND SHOP DRAWING PLANS. THE CONTRACT FLANS SUBSEQUENT RECONSTRUCTION AND SHOP DRAWING PLANS. THE CONTRACTOR SHALL VERIFY DIMENSIONS, INCLUDING THICKNESSES OF PARTS, WITH FIELD MEASUREMENTS PRIOR TO ORDERING MATERIALS OR FABRICATING STEEL. ALL PLAN DIMENSIONS ARE FOR A NORMAL TEMPERATURE OF 60 DEG F. LAYOUT DIMENSIONS ARE HORIZONTAL MEASUREMENTS AND DO NOT NECESSARILY REFLECT REVISIONS.

PLANS OF EXISTING STRUCTURE

PLANS AND SHOP DRAWINGS OF THE EXISTING STRUCTURE ARE AVAILABLE AS AN AID TO THE CONTRACTOR AND SHALL BE USED TO SUPPLEMENT DETAILS NOT SHOWN ON THE PLANS. THE COMPLETENESS OF THESE DRAWINGS IS NOT GUARANTEED AND NO RESPONSIBILITY IS ASSUMED BY KYTC FOR THEIR ACCURACY. AS-BUILT PLANS AND SHOP DRAWINGS INCLUDE:

I-65	SB_AS-BUILT_ORBP	056B0002141L	REHAB	PLANS
I-65	SUBSTRUCTURE AS-BUILT PLANS	DN 14525		
I-65	SUPERSTRUCTURE AS-BUILT PLANS	DN 14744		
I-65	SUPERSTRUCTURE SHOP PLANS	DN 14744		
I-65	OVER OHIO RIVER ANCHOR ASSEMBLY @ PIER 6	DN 26658		

EXISTING STEEL REINFORCEMENT

PAYMENT FOR CUTTING, BENDING, SPLICING AND CLEANING EXISTING REINFORCING BARS SHALL BE INCLUDED IN THE APPROPRIATE PAY ITEM.

SAWCUTTING

PRIOR TO THE REMOVAL OF THE EXISTING CONCRETE MASONRY, CUT THE SURFACE WITH A CONCRETE SAW TO THE DEPTH NOTED ON THE PLANS OR ONE INCH TO FACILITATE A NEAT LINE. PAYMENT FOR CUTTING CONCRETE SHALL BE INCIDENTAL TO THE APPROPRIATE PAY ITEM.

BOL TS

ALL STRUCTURAL BOLTS SHALL BE 7/8" DIA. ASTM F3125 GRADE 325 HIGH STRENGTH BOLTS EXCEPT AS NOTED.

TO REINSTALL GENERATION 3 RETROFIT, STRUCTURAL BOLTS SHALL BE 1" DIA. ASTM F3125 BOLTS GRADE 490 HIGH STRENGTH BOLTS.

ALL BOLTS SHALL BE GALVANIZED.

CONCRETE REMOVAL

PERFORM WORK CAREFULLY DURING CONCRETE REMOVAL TO PROTECT PORTIONS OF SUCH SYSTEMS THAT ARE TO BE SALVAGED AND INCORPORATED INTO THE PROPOSED STRUCTURE. ALL REMOVAL SHALL BE TO NEAT SAW CUT LINES. FEATHER EDGES WILL NOT BE PERMITTED. SAW CUT BOUNDARIES OF PROPOSED CONCRETE REMOVAL 1 INCH DEEP.

THE CONTRACTOR MAY REMOVE CONCRETE BY CUTTING AND BY MEANS OF HAND OPERATED PNEUMATIC HAMMERS EMPLOYING POINTED OR BLUNTED CHISEL TYPE TOOLS. DO NOT PLACE PNEUMATIC HAMMERS IN DIRECT CONTACT WITH REINFORCING STEEL THAT IS TO BE RETAINED IN THE PROPOSED STRUCTURE. REMOVE CONCRETE TO LIMITS AS SHOWN ON THE PLANS. LEAVE EXISTING REINFORCING STEEL IN PLACE AS SHOWN ON THE PLANS.

PRIOR TO NON-SHRINK GROUT PLACEMENT, ABRASIVELY CLEAN JOINT SURFACES AND PRIOR TO NON-SHRINK GROUT PLACEMENT, ABRASIVELY CLEAN JOINT SURFACES AND EXISTING EXPOSED REINFORCEMENT TO REMOVE LOOSE AND DISINTEGRATED CONCRETE AND LOOSE RUST. THOROUGHLY CLEAN THE JOINT SURFACE AND EXPOSED REINFORCEMENT OF ALL DIRT, DUST, RUST OR OTHER FOREIGN MATERIAL BY THE USE OF WATER, AIR PRESSURE OR OTHER METHODS THAT PRODUCE SATISFACTORY RESULTS. EXISTING REINFORCING STEEL DOES NOT HAVE TO HAVE A BRIGHT STEEL FINISH, HOWEVER, REMOVE ALL PACK AND LOOSE RUST. IN ADDITION, PREPARE THE EXISTING SURFACE PER THE NON-SHRINK GROUT MANUFACTURER'S RECOMMENDATIONS PRIOR TO APPLICATION.

REINFORCING BARS WHICH ARE SHOWN ON THE PLANS AS REMAINING AND WHICH ARE DAMAGED BY THE CONTRACTOR'S OPERATIONS SHALL BE REPLACED WITH NEW EPOXY COATED BARS OF THE SAME SIZE AND SHAPE, AS APPROVED BY THE ENGINEER. NO ADDITIONAL PAYMENT WILL BE MADE FOR THOSE BARS.

PAYMENT FOR THIS WORK SHALL BE INCLUDED IN THE LUMP SUM CONTRACT PRICE FOR CONCRETE REPAIR - PIER 6L DS MASONRY PLATE.

NON-SHRINK GROUT

THE FOLLOWING PRODUCTS HAVE BEEN PRE-APPROVED, AND ARE ON THE LIST OF APPROVED MATERIALS, FOR THE NON-SHRINK GROUT AT PIER 6 DOWNSTREAM:

- TAMMSGROUT SUPREME. HIGH STRENGTH NON-SHRINK GROUT BY THE EUCLID CHEMICAL COMPANY
- MASTERFLOW 928 BY MASTERFLOW BASF
- 1107 ADVANTAGE GROUT BY DAYTON SUPERIOR CHEMICALS
- USE PEA GRAVEL AGGREGATE PER MANUFACTURER'S REOMMENDATIONS.
- THE GROUT SHALL PROVIDE FULL CONTACT WITH THE MASONRY PLATE AND BE PLACED PER THE MANUFACTURER'S RECOMMENDATIONS. CURE PER MANUFACTURER'S RECOMMENDATIONS FOR BASE PLATE APPLICATIONS AND HEAVY LOADS. THE SELECTED MATERIAL SHALL BE RECOMMENDED FOR BASE PLATE APPLICATIONS BY THE MANFACTURER AND HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI BEFORE LIVE LOAD IS ALLOWED TO BE APPLIED. THE GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 5000 PSI AT 28 DAYS.

REMOVE STEEL

ALL EXISTING STEEL THAT IS REMOVED AND NOT REUSED IN THE COMPLETED STRUCTURE SHALL BECOME THE PROPERTY OF THE CONTRACTOR AND SHALL BE REMOVED FROM THE BRIDGE SITE.

PLAN SET A AND PLAN SET B COORDINATION

WORK ON THE TRUSS BEARINGS AND FINGER JOINT REPLACEMENT CAN BE STAGED CONCURRENTLY BY THE CONTRACTOR IF DESIRED, AS LONG AS THE FINGER JOINT REPLACEMENT RESULTS IN PROPER PHYSICAL ALIGNMENT AND FINAL ELEVATIONS. IF THE FINAL RESULTS DO NOT COMPLY WITH THIS REQUIREMENT, CORRECTIONS MUST BE MADE AS APPROPPRIATE AT THE CONTRACTOR'S EXPENSE.

PINS

PINS SHALL BE 10 1/2" DIAMETER STANDARD RECESSED PINS WITH TWO STANDARD HEXAGONAL RECESSED NUTS AND TWO BRONZE WASHERS. RECESSED PINS AND RECESSED PIN NUTS SHALL BE ASTM A668 CLASS C; BRONZE WASHERS SHALL BE ASTM B22-14.

PINS ARE NON-REDUNDANT STEEL TENSION MEMBERS (NSTM), CHARPY V-NOTCH IMPACT TEST THE PINS IN ACCORDANCE WITH ASTM A673, P FREQUENCY. THE SAMPLES SHALL WITHSTAND AN IMPACT OF 25 FT-LBS AT 40 DEGREES F.

GALVANIZE THE PINS IN ACCORDANCE WITH ASTM A123. DO NOT EXCEED A GALVANIZING THICKNESS OF 0.03125 INCH ON THE PIN.

TURN PINS TO THE SPECIFIED DIMENSIONS. ENSURE THEY ARE SMOOTH. STRAIGHT. AND FREE FROM FLAWS.

FORGE AND ANNEAL PINS MORE THAN 9 INCHES IN DIAMETER.

PINS NOT MEETING THESE REQUIREMENTS MAY BE REJECTED AT THE ENGINEER'S DISCRETION. REPLACE PINS AT NO ADDITIONAL EXPENSE TO THE CABINET.

PROVIDE TWO PILOT NUTS AND TWO DRIVING NUTS FOR THE PIN SIZE SHOWN IN THE PLANS IN ACCORDANCE WITH SECTION 607.03.09 OF THE STANDARD SPECIFICATIONS.

PIN HOLES

INTERIOR CONDITION.

LINE BORE PIN HOLES TO BE: 1) TRUE TO DETAILED DIMENSIONS, 2) SMOOTH, AND 3) STRAIGHT AT RIGHT ANGLES WITH THE AXIS OF THE MEMBER AND PARALLEL WITH EACH OTHER. PINS AND PIN HOLES SURFACE FINISH SHALL MEET ANSI 125.

CHECK MEASUREMENTS

PRIOR TO THE FABRICATION OF THE PINS, THE CONTRACTOR SHALL MAKE CHECK MEASUREMENTS IN THE FIELD AND MAKE ANY ADJUSTMENTS NECESSARY TO MEET THE REQUIRED CLEARANCES AND TO FIT THE PROPOSED WORK TO EXISTING CONDITIONS. THESE FIELD VERIFICATION MEASUREMENTS SHALL BE SUBMITTED TO THE ENGINEER PRIOR TO SHOP DRAWING APPROVAL.

DRAWINGS SHOW GENERAL FEATURES OF DESIGN ONLY. SHOP DRAWINGS SHALL BE MADE IN ACCORDANCE WITH THE SPECIFICATIONS, SUBMITTED AND APPROVAL OBTAINED BEFORE FABRICATION IS STARTED.

TEMPORARY SUPPORTS

A PIN AND LINK DETAIL IS USED TO CONNECT THE SUPERSTRUCTURE TO A SUPPORTING BEARING ANCHORED TO THE SUBSTRUCTURE. HOLES IN THE SUPERSTRUCTURE GUSSET AND BEARING MAIN PLATE ARE CONNECTED USING A STEEL LINK PLATE. LARGE STEEL PINS THROUGH THE PLATE MUST SUPPORT MAXIMUM AND MINIMUM COMBINATIONS OF DEAD LOAD AND LIVE LOAD WHILE ALLOWING THE SUPERSTRUCTURE TO MOVE LONGITUDINALLY.

WHEN REMOVING THE PINS AND BORING THE EXISTING HOLES FOR THE NEW PINS, TEMPORARY SUPPORTS ARE REQUIRED TO UNLOAD THE LINK PLATE. ONCE INSTALLED, THE INTENT IS FOR DEAD LOAD AND LIVE LOAD PRODUCED ON THE SPAN TO BYPASS THE LINK PLATE AND TRANSFER TO THE TEMPORARY SUPPORTS. SEE SPECIAL NOTE FOR TEMPORARY SUPPORTS.

NEW STEEL PAINTING

NEW PINS SHALL BE PAINTED IN THE FIELD AFTER COMPLETION OF THE REPAIRS. NEW LINK PLATE TO RECEIVE A SHOP PRIME COAT, THEN FIELD APPLIED FOR INTERMEDIATE AND FINAL COAT. THE COST IS TO BE INCLUDED WITH THE APPROPRIATE BID ITEMS.

TOUCH-UP PAINTING

ALL AREAS OF NEW OR EXISTING STRUCTURAL STEEL ON WHICH THE PAINT HAS BEEN DAMAGED BY THE CONTRACTOR WITH WELD BURNS OR BY OTHER MEANS DURING CONSTRUCTION OR AFTER FINAL PAINTING SHALL BE WIRE BRUSH CLEANED AND SPOT PAINTED AS DIRECTED BY THE ENGINEER. THE COST IS TO BE INCLUDED WITH THE APPROPRIATE BID ITEMS.

FOR PIN HOLES LARGER THAN 9 INCHES IN DIAMETER, LONGITUDINALLY BORE A 2 INCH HOLE THROUGH THE CENTER AFTER FORGING HAS COOLED BELOW THE CRITICAL RANGE AND BEFORE THE FORGING IS ANNEALED. REJECT PINS SHOWING A DEFECTIVE

PROVIDE A PIN HOLE DIAMETER THAT DOES NOT EXCEED THAT OF THE PIN BY MORE THAN 0.0625 INCH AS MEASURED ON THE GALVANIZED SURFACES. DO NOT EXCEED A GALVANIZING THICKNESS OF 0.03125 INCH ON THE INSIDE SURFACE OF THE PIN HOLE WHERE IT CONTACTS THE PIN.

THIS WORK CONSISTS OF PAINTING THE NEW STEEL PIECES FOR THE TRUSS BEARINGS AT PIER IL AND PIER GL IN ACCORDANCE WITH SECTION 607 OF THE STANDARD SPECIFICATIONS. MATCH THE NEW PAINT FINISH COAT COLOR WITH THE CURRENT COLOR OF THE STRUCTURE.

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TRUSS PIN REPLACEMENT - PIER IL US TRUSS PIN REPLACEMENT - PIER IL DS TRUSS PIN REPLACEMENT - PIER 6L US TRUSS PIN REPLACEMENT - PIER 6L DS

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THE WORK FOR EACH OF THESE ITEMS CONSISTS OF REMOVAL AND REPLACEMENT OF THE UPPER AND LOWER PIN THROUGH A LINK PLATE THAT CONNECTS THE SUPERSTRUCTURE TO THE SUPPORTING BEARING. PART OF THE PIN REPLACEMENT WORK AT PIER IL SHALL INCLUDE FABRICATION AND INSTALLATION OF NEW LINK PLATE ASSEMBLIES PRIOR TO LINE BORING.

WHEN TEMPORARY SUPPORTS HAVE BEEN INSTALLED TO TRANSFER LOAD OFF THE LINK PLATE ASSEMBLY, THE PIN HOLES SHALL BE LINE BORED TO REMOVE EXISTING HOLE DISTORTION, NEW LINK PLATE ASSEMBLIES AT PIER IL SHALL BE ALIGNED WITH THE EXISTING PLATES AND LINE BORED WITH THE PLATES.

ADDITIONAL WORK AT PIER 1L AND PIER 6L US INCLUDES CLEANING AND PAINTING THE SUPPORTING BEARING, GUSSET PLATE CONNECTION AND GENERATION 2 RETROFIT.

ADDITIONAL WORK AT PIER 6L DS INCLUDES CLEANING AND PAINTING THE SUPPORTING BEARING, GUSSET PLATE CONNECTION AND GENERATION 3 RETROFIT. WORK AT PIER GL DS ALSO INCLUDES GENERATION 3 RETROFIT REMOVAL AND REINSTALLATION, AND REMOVAL AND DISPOSAL OF GENERATION 2 RETROFIT.

THE LUMP SUM CONTRACT PRICE FOR EACH OF THESE ITEMS SHALL BE FULL PAYMENT FOR MATERIALS, TOOLS, EQUIPMENT, LABOR, ACCESS AND INCIDENTALS TO COMPLETE THE WORK. TEMPORARY SUPPORT REMOVAL, REMOVAL/REINSTALL OF EXISTING STRUCTURE CONFLICTS IN THE WORK AREA, PROOF LOAD TESTING OF GENERATION 2 ANCHOR RODS, AND GENERATION 3 RETROFIT REINSTALL IS CONSIDERED INCIDENTAL FOR PAYMENT IN EACH APPROPRIATE PAY ITEM.

CONCRETE REPAIR - PIER 6L DS MASONRY PLATE

THE WORK FOR THIS ITEM CONSISTS OF REMOVAL AND REPLACEMENT OF THE CONCRETE AROUND AND UNDER THE EXISTING BEARING MASONRY PLATE IN ACCORDANCE WITH THE PLANS AND SPECIFICATIONS.

VERTICAL SUPPORT SHALL BE PROVIDED AS SHOWN ON THE PLANS TO ENSURE THE BEARING IS NOT SUBJECT TO POSITIVE LIVE LOAD PRODUCED ON THE SPAN.

NON-SHRINK GROUT SHALL BE PLACED AS SHOWN ON THE PLANS IN ACCORDANCE WITH THE SPECIFICATIONS. FULL CONTACT WITH THE BOTTOM OF THE PLATE SHALL BE ACHIEVED.

THE LUMP SUM CONTRACT PRICE FOR THIS ITEM SHALL BE FULL PAYMENT FOR MATERIALS, TOOLS, EQUIPMENT, LABOR, ACCESS AND INCIDENTALS TO COMPLETE THE WORK, TEMPORARY SUPPORT INSTALLATION, REMOVAL/REINSTALL OF EXISTING STRUCTURE CONFLICTS IN THE WORK AREA AS PART OF THIS WORK IS CONSIDERED INCIDENTAL FOR PAYMENT IN THE APPROPRIATE BID ITEM.

STEEL FOR TEMPORARY HOLD DOWN

PROVIDE TEMPORARY HANGER THREADED RODS MEETING ASTM F1554 GRADE 55. PROVIDE LENGTH OF ROD TO ALLOW TENSIONING EQUIPMENT. THREADS MUST BE COMPATIBLE WITH EXISTING ANCHOR ROD.

PROVIDE RODS WITH LONGITUDINAL CHARPY V-NOTCH IMPACT VALUES OF 30 FT-LBS AT 10 DEG F. HEAT TREAT THE STEEL, IF NECESSARY, TO MEET THE CHARPY V-NOTCH IMPACT REQUIREMENTS. THE ENGINEER WILL REJECT RODS WITH NOTCHES, NICKS, OR WELDS.

FURNISH A SAMPLE 45 INCHES LONG SAW CUT FROM EACH BAR LENGTH TO BE SUPPLIED FOR HANGER ROD FABRICATION. A CABINET REPRESENTATIVE MUST WITHNESS THE REMOVAL OF THE TEST SAMPLE FROM EACH BAR LENGTH SUPPLIED. THE CABINET WILL USE THESE SAMPLES FOR TENSILE STRENGTH, YIELD STRENGTH, AND IMPACT TESTING.

REDUCE THE SAMPLE LENGTH TO 8 INCHES (FOR IMPACT TESTING ONLY) IF A CERTIFIED MILL TEST REPORT TRACEABLE TO THE MATERIAL IS PROVIDED. MATCH MARK EACH BAR SAMPLE AND CORRESPONDING REMAINDER BY STENCILING IN THE END CROSS SECTION.

ACCEPTANCE OF ROD MATERIAL WILL BE BASED ONLY ON THE CABINET TESTING AND TRACEABLE CERTIFIED MILL TEST REPORTS.

PROVIDE HEAVY HEX NUTS AND COUPLING NUTS FOR THE TEMPORARY HANGER RODS ACCORDING TO ASTM A194 GRADE 2H, AND WASHERS ACCORDING TO ASTM F436. THREADS MUST BE COMPATIBLE WITH EXISTING 1 $\frac{3}{4}$ " ANCHORS.

PROVIDE OTHER STEEL FOR THE TEMPORARY SUPPORTS ACCORDING TO ASTM A709 GRADE 50.

PRELOAD HANGER ROD TO A 70 KIP TENSION USING HOLLOW RAM HYDRAULIC JACK WHICH COUPLES DIRECTLY TO THE END OF THE ROD. WHEN THE PRELOADING IS REACHED, TIGHTEN THE HANGER ROD NUT TIGHTLY AGAINST THE BEARING PLATE AND RELEASE THE LOAD FROM THE JACK. ENSURE THE HANGER ROD NUT PREVENTS THE STEEL FROM RELAXING BACK TO ITS ORIGINAL LENGTH SO THAT THE ROD IS PRESTRESSED.

WIND LOCK REPAIR AT PIER 6L

THIS WORK SHALL CONSIST OF REMOVAL AND REPLACEMENT IN KIND OF THE TWO WIND LOCK BEAM ASSEMBLIES AT PIER 6L AS SHOWN ON THE PLANS, NEW BEAM ASSEMBLIES INCLUDE BEAMS, PLATES, SHIMS, BOLTS, AND PAINT AS SHOWN ON THE PLAN SHEET. PAYMENT FOR ALL LABOR, MATERIALS, AND INCIDENTALS SHALL BE INCLUDED IN THE UNIT BID PRICE FOR "STEEL REPAIR - WIND LOCK AT PIER 6L".

CONTRACTOR SUBMITTALS

WHERE REQUIRED BY THE PLANS AND SPECIFICATIONS, CONTRACTOR SHALL SUBMIT DESCRIPTIVE INFORMATION THAT WILL ENABLE ENGINEER TO DETERMINE WHETHER CONTRACTOR'S PROPOSED MATERIALS, EQUIPMENT, AND WORK METHODS ARE IN GENERAL CONFORMANCE WITH THE PLANS AND SPECIFICATIONS.

THE CONTRACTOR PERFORMING THE LINE BORING SHALL HAVE 5 YEARS MINIMUM EXPERIENCE CONDUCTING ON-SITE LINE BORING TO PRIMARY STRUCTURAL MEMBERS OR TO EQUIVALENT INDUSTRIAL EQUIPMENT OR HEAVY MACHINERY. THE LINE BORING CONTRACTOR MUST HAVE COMPLETED A MINIMUM OF 10 IN-SITU LINE BORING OPERATIONS THAT ARE SIMILAR IN NATURE TO THAT SPECIFIED OVER THE PAST 5 YEARS. AT LEAST 15 WORKING DAYS BEFORE BEGINNING THE LINE BORING OPERATION, FURNISH THE ENGINEER A WRITTEN PLAN FOR THE LINE BORING OPERATION. INCLUDE EVIDENCE SATISFACTORY TO THE ENGINEER THAT THE PLANNED OPERATIONS CONFORM TO THE DECONDENT IN THE CONTRACT POCHNENTS. TO THE REQUIREMENTS IN THE CONTRACT DOCUMENTS. A MINIMUM OF 15 WORKING DAYS PRIOR TO THE PRE-CONSTRUCTION MEETING, SUBMIT TO THE ENGINEER A REPORT DOCUMENTING THE EXPERIENCE OF THE CONTRACTOR'S PERSONNEL WITH SIMILAR WORK. INCLUDE THE PROJECTS WORKED ON WITH THE DATE, LOCATION AND NUMBER OF LINE BORING WORK, ALONG WITH CONTACT NAMES, CURRENT PHONE NUMBERS AND E-MAIL ADDRESSES.

THE CONTRACTOR SHALL SUBMIT SEQUENCES. TECHNIQUES AND PROCEDURES OF CONSTRUCTION INCLUDING, BUT NOT LIMITED TO, LABOR, MATERIALS, TEMPORARY STRUCTURES, TOOLS, CONSTRUCTION EQUIPMENT, AND ALL INCIDENTAL OR TEMPORARY DEVICES REQUIRED TO ACCOMPLISH THE RESULT INTENDED BY THIS CONTRACT.

PIN REPLACEMENT

A PIN AND LINK DETAIL IS USED TO CONNECT THE SUPERSTRUCTURE TO A BEARING ANCHORED TO THE SUBSTRUCTURE. A STEEL LINK PLATE CONNECTS THE HOLE IN THE TRUSS LOWER CHORD GUSSET PLATE TO THE HOLE IN THE BEARING STEEL PLATE. LARGE STEEL PINS THROUGH THE PLATE MUST SUPPORT MAXIMUM AND MINIMUM COMBINATIONS OF DEAD LOAD AND LIVE LOAD WHILE ALLOWING THE TRUSS SPAN TO MOVE LONGITUDINALLY. THE STEEL LINK PLATES ALSO PROVIDE RESTRAINT FOR UPLIET.

PIN AND LINK PLATE ASSEMBLIES ARE NON-REDUNDANT STEEL TENSION MEMBERS WHOSE FAILURE WOULD RESULT IN COLLAPSE OF THE BRIDGE OR CAUSE IT TO BE UNABLE TO PERFORM ITS INTENDED FUNCTION. THE ORIGINAL PINS AND PIN HOLES HAVE DETERIORATED TO A POINT WHERE REPLACEMENT IS ADVISED TO MAINTAIN INTENDED SAFETY AND PERFORMANCE.

WORK AT PIER IL CONSISTS OF REPLACING THE STEEL LINK PLATE ASSEMBLIES ALONG WITH THE UPPER AND LOWER PIN REPLACEMENTS. WORK AT PIER 6L CONSISTS OF UPPER AND LOWER PIN REPLACEMENT ONLY. BORE GUSSET AND BEARING PLATE HOLES WITH THE NEW LINK PLATE ASSEMBLY IN PLACE AT PIER IL; AND WITH THE EXISTING LINK PLATE ASSEMBLY IN PLACE AT PIER 6L. BORE ALL PIN HOLES TO A SLIGHTLY LARGER DIAMETER TO REMOVE EXISTING DISTORTION. NEW HOLES SHALL BE AS SHOWN IN THE PLANS FOR NEW PIN DIAMETERS ONE-HALF INCH LARGER THAN THE ORIGINAL PLANS.

BORE HOLES AS SHOWN IN THE PLANS USING LINE BORING FIELD MACHINING. ENSURE THE CENTERLINES OF BORES EACH SIDE OF THE CENTERLINE TRUSS ARE COLINEAR, ROUND AND CONCENTRIC. BORING EQUIPMENT SHALL FIT WITHIN THE SPACE CONSTRAINTS OF THE JOB SITE AND WORK AROUND ANY OBSTRUCTIONS THAT MAY BE IN THE WAY.

MAKE PRECISE MEASUREMENTS OF PIN LOCATIONS AND LINK PLATE PLUMBNESS AND OTHER MEASUREMENTS AND ALIGNMENT CONTROL ESSENTIAL TO LINE BORING ON-SITE. SUBMIT THE FIELD DATA FOR ALL 4 BEARINGS FOR REVIEW PRIOR TO COMMENCING MACHINING OPERATIONS.

DETAILS ARE NOT PROVIDED IN THE PLANS TO LOCK THE BRIDGE DOWN TO THERMAL MOVEMENT. SUGGEST THE LOWER PIN BE REPLACED FIRST SINCE THE BEARING PINNED CONNECTION IS FIXED TO THE SUBSTRUCTURE AND NOT SUBJECT TO THERMAL MOVEMENT. INSTALL THE NEW LINK PLATE ASSEMBLY ROTATED DOWN AT PIER 1L. ONCE THE UPPER PIN IS REMOVED, ROTATE THE NEW LINK PLATE UP FOR THE TOP PIN HOLE BORING OPERATION.

THE LINK PLATE WAS DESIGNED TO BE VERTICAL AT 60° F. RECORD THE TEMPERATURE AND LINK PLATE INCLINATION IMMEDIATELY BEFORE COMMENCING BOTTOM PIN REMOVAL. THE TOP PIN MOVES ON A 30 INCH CIRCULAR ARC ROTATION ABOUT THE LOWER PIN CENTERLINE. ROTATE THE LINK PLATE TO ALIGN WITH THE GUSSET PLATE PIN HOLE. ENSURE THE PLATE HOLES ARE CONCENTRIC AND SECURED BEFORE INITIATION PROPIND OPERATION INITIATING BORING OPERATION.

TEMPERATURE ADJUSTMENT AT PIER IL EXPANSION JOINT IS \pm 5/16 INCH PER 10° F. TEMPERATURE ADJUSTMENT AT PIER GL EXPANSION JOINT IS \pm 5/8 INCH PER 10° F. THE SUGGESTED PIN REPLACEMENT CONCEPT IS PRESENTED AS ONE METHOD TO ACCOUNT FOR THERMAL EFFECTS ON THE PIN REPLACEMENT WORK. CONTRACTOR IS RESPONSIBLE FOR THE COMPLETE LINE BORING PLAN AND SHALL SUBMIT APPROPRIATE CONSTRUCTION PROCEDURES, DETAILS AND CALCULATIONS TO DESCRIBE THE PROCESS.

SUBMIT SEQUENCES, TECHNIQUES, AND PROCEDURES OF CONSTRUCTION INCLUDING BUT NOT LIMITED TO LABOR, MATERIALS, TEMPORARY STRUCTURES, CONSTRUCTION EQUIPMENT, AND ALL INCIDENTAL OR TEMPORARY DEVICES REQUIRED TO ACCOMPLISH THE RESULT INTENDED BY THIS CONTRACT. SUBMIT GEOMETRIC CONTROL PROCEDURES AND FIELD DATA FOR ALL BEARINGS FOR REVIEW PRIOR TO COMMENCING MACHINING OPERATIONS.

PAYMENT FOR ALL LABOR, ENGINEERING, MATERIALS, TOOLS, EQUIPMENT, JACKING SYSTEM, ACCESS AND INCIDENTALS TO FURNISH AND INSTALL JACKING SUPPORTS AND REMOVE WHEN WORK IS COMPLETED SHALL BE INCLUDED IN THE APPROPRIATE BID ITEM.

JACKING SUPPORT

THIS WORK CONSISTS OF FURNISHING ALL LABOR, TOOLS, AND EQUIPMENT FOR JACKING AND SUPPORTING THE EXISTING BEAM WHILE REMOVING THE LINK PINS AND BORING HOLES FOR NEW REPLACEMENT PINS. THE CONTRACTOR IS RESPONSIBLE FOR THE COMPLETE DESIGN OF THE BRIDGE LIFTING PROCEDURES AND THE MATERIALS USED. FURNISH AND PLACE ALL BRACING, BLOCKING, TEMPORARY STRUCTURAL STEEL, SHIMS, WEDGES, HYDRAULIC JACKS, AND ANY OTHER MATERIALS AND EQUIPMENT NECESSARY FOR PROPER EXECUTION OF THE WORK.

THE CONTRACTOR SHALL DEVELOP A PLAN AND SUPPORTING CALCULATIONS FOR JACKING, BLOCKING, AND SUPPORTING BEAMS. ALL JACKS AND TEMPORARY SUPPORT SYSTEMS SHALL BE DESIGNED TO SUSTAIN TRAFFIC LOADINGS, DEAD LOAD, TEMPORARY CONSTRUCTION LOADS, AND ALL OTHER ANTICIPATED LOADING DURING WORK REQUIRING THE JACKING AND BLOCKING OF BEAMS. THE DESIGN SHALL BE IN ACCORDANCE WITH CURRENT AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THE CONTRACTOR SHALL SUBMIT DETAILS AND CALCULATIONS OF THE PROPOSED JACKING SYSTEMS AND TEMPORARY SUPPORT PROCEDURES FOR REVIEW BY THE ENGINEER BEFORE COMMENCING WORK. THE CONTRACTOR'S JACKING PLANS AND PROCEDURES SHALL BE DESIGNED AND SEALED BY A KENTUCKY LICENSED PROFESSIONAL ENGINEER.

THE CONTRACTOR SHALL MONITOR THE JACKING PROCEDURE TO ENSURE THAT JACKING DOES NOT CAUSE DAMAGE AT ANY LOCATION IN THE SPANS. IF THERE IS ANY EVIDENCE OF DAMAGE OR UNUSUAL SITUATION OCCURRING DURING THE JACKING OPERATIONS AT ANY LOCATION ALONG THE SPAN, THE CONTRACTOR SHALL TAKE CORRECTIVE ACTIONS AND NOTIFY THE ENGINEER IMMEDIATELY.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGE TO THE STRUCTURE CAUSED BY JACKING. NO STRUCTURAL ELEMENTS SHALL BE REMOVED FROM THE EXISTING STRUCTURE WITHOUT PRIOR APPROVAL FROM THE ENGINEER.

SIZE JACKS FOR AT LEAST 150 PERCENT OF THE CALCULATED LIFTING LOAD. THE MINIMUM JACK CAPACITY SHALL BE AS NOTED IN THE PLANS. THE JACKS AND THE JACKING SUPPORTS SHALL BE PLUMB AND PLACED AT LOCATIONS OF LEVEL AND SOUND CONCRETE, MONITOR LATERAL DEFLECTION OF THE JACKING SYSTEM TO ENSURE THAT THE JACKING SYSTEM REMAINS PLUMB. JACKING SHALL BE PERFORMED UNDER ACTIVE TRAFFIC CONDITIONS.

BEAMS TO BE JACKED AND BLOCKED SHALL NOT BE JACKED MORE THAN $\frac{1}{8}$ INCH OR AS DIRECTED BY THE ENGINEER. THE DIFFERENCE IN ELEVATION BETWEEN ADJACENT BEAMS DURING JACKING AND BLOCKING SHALL NOT BE GREATER THAN $\frac{1}{8}$ INCH. SUITABLE GAUGES FOR THE MEASUREMENT OF SUPERSTRUCTURE MOVEMENT SHALL BE FURNISHED BY THE CONTRACTOR.

BEARING STIFFENERS ARE TO BE ADDED TO THE EXISTING BEAM DIRECTLY ABOVE THE JACKING POINT TO EFFECTIVELY TRANSFER THE REACTION TO THE JACKS. PROVIDE JACKS WITH ABILITY TO PROVIDE A LOCKING NUT SYSTEM TO RETAIN THE LOADS WITHOUT RELYING ON MAINTAINING THE HYDRAULIC PRESSURE FOR THE ENTIRE TIME THAT THE LOAD IS ENGAGED. PROVIDE CHANNEL SHIMS FOR SUPPLEMENTARY SUPPORT DURING PIN REPLACEMENT WORK.

PAYMENT FOR ALL LABOR, ENGINEERING, MATERIALS, TOOLS, EQUIPMENT, JACKING SYSTEM, ACCESS AND INCIDENTALS TO FURNISH AND INSTALL JACKING SUPPORTS AND REMOVE WHEN WORK IS COMPLETED SHALL BE INCLUDED IN THE APPROPRIATE BID ITEM.

LEAD PAINT

OF CONNECTIONS.

RESIDUAL LEAD PAINT MAY STILL BE ON THE BRIDGE EVEN AFTER PREVIOUS SANDBLASTING AND PAINTING OF THE BRIDGE. CONSEQUENTLY, THE CONTRACTOR IS ADVISED TO TAKE ALL NECESSARY PROTECTIVE MEASURES WHEN REMOVING, CUTTING, OR PERFORMING ANY OTHER ACTIONS ON THE EXISTING STEEL ESPECIALLY IN AREAS

DAMAGE TO THE STRUCTURE

THE CONTRACTOR SHALL BEAR FULL RESPONSIBILITY AND EXPENSE FOR ANY AND ALL DAMAGE TO THE STRUCTURE, INCLUDING TRUSS MEMBERS, DURING THE REPAIR AND RETROFIT WORK; EVEN TO THE REMOVAL AND REPLACEMENT OF TRUSS MEMBERS AND FALLEN SPANS, SHOULD THE DAMAGE RESULT FROM THE CONTRACTOR'S ACTIONS.

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Phone: (502) 339-355

28935





PIER 6L ELEVATION

LOOKING AHEAD STATION PLATFORM NOT SHOWN FOR CLARITY

PIER 6L JACKING SUPPORT

THE TRUSS DEAD LOAD PRODUCES UPLIFT AT THE PIER 6L PIN AND LINK BEARINGS. THE TRUSS UPLIFT IS TO BE REMOVED OFF THE LINK PLATES WHEN THE ANCHOR RODS ARE TIGHTENED TO PRELOAD.

THE FLOORBEAM IS TO BE JACKED TO REMOVE THE FLOORBEAM DEADLOAD OFF THE LINK PLATE TO TRANSFER TO THE JACKING SUPPORT. POSITIVE MOT (MAINTENANCE OF TRAFFIC) LIVE LOAD PRODUCED ON THE END SPAN MUST ALSO BYPASS THE LINK PLATE TO TRANSFER TO THE JACKING SUPPORT. SEE GENERAL NOTES.

PIER 6L I	LOAD TAE	BLE (PER	BEARING)
MIN. LOAD	MAX. LOAD	NUMBER	MIN. JACK
1.5(DL+LL+I)	1.5(DL+LL+I)	OF	CAPACITY
(UPLIFT)	(JACKING)	JACKS	(TONS)
(KIP)	(KIP)		
-270	32	1	16

NOTES

- 1. NEW STRUCTURAL STEEL SHALL CONFORM TO ASTM A709 GRADE 50.
- 2. NEW STEEL SHALL BE PRIMED AND PAINTED IN THE SHOP.
- 3. STIFFENER TIGHT FIT AT TOP AND BOTTOM OF L8×4 TO MEET THE REQUIREMENTS OF SECTION 607.03.08 (1) OF THE STANDARD SPECIFICATIONS.
- 4. GRIND UPPER AND LOWER CORNERS OF L8×4 TO MISS THE FLOORBEAM FLANGE TO WEB WELDS.
- 5. SEE GENERAL NOTES FOR BOLTS.
- 6. JACKING STIFFENERS TO REMAIN IN PLACE WHEN WORK IS COMPLETED.
- 7. PIPE HANDRAIL MAY BE REMOVED IF NECESSARY TO ACCESS WORK AND SHALL BE REPLACED AS REMOVED WHEN WORK IS COMPLETED.

PLAN SET

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DE DE ROUTE I-65	PARTMEN JEF		HIGH ON SING RIVER	IMCR	y YS
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NOTES FOR FIELD PAINTING US / DS TRUSS BEARINGS AT PIERS 1L AND 6L

FOLLOWING COMPLETED REPAIR OF THE TRUSS BEARINGS AT PIERS IL AND 6L, CLEAN AND PAINT THE ENTIRE TRUSS BEARING ASSEMBLY IN ACCORDANCE WITH SECTION 607 OF THE STANDARD SPECIFICATIONS. LEVEL OF CLEANING TO BE AN SSPC-SP15 (COMMERCIAL GRADE POWER TOOL CLEANING). ALL POWER TOOLS SHALL BE EQUIPPED WITH VACUUM SHROUDS AND FITTED WITH HEPA FILTERS AT THEIR AIR EXHAUSTS. MAINTAIN AND OPERATE ALL VACUUM SHROUDED POWER TOOLS TO COLLECT GENERATED DEBRIS.

TRUSS BEARINGS SHALL RECEIVE A THREE COAT PAINT SYSTEM IN ACCORDANCE WITH SECTION 607 OF THE STANDARD SPECIFICATIONS. MATCH THE NEW PAINT FINISH COAT COLOR WITH THE CURRENT COLOR OF THE STRUCTURE. THE COST IS TO BE INCLUDED WITH THE APPROPRIATE BID ITEMS. WORK WITH NEW STEEL PAINTING GENERAL NOTE.

© TRUSS © BEARING _____ LEAVE EXISTING RODS IN PLACE (ø

TRUSS BEARING WITH GEN 2 RETROFIT SHOWN

NOTES

- ALL PIER 6L DS GENERATION 2 RETROFITS MAY BE PERMANENTLY REMOVED PRIOR TO TESTING THE ANCHOR RODS. REINSTALLATION IS NOT REQUIRED.
- 2. ANCHOR RODS SHALL BE PROTECTED DURING THIS REMOVAL WORK.
- 3. GENERATION 3 RETROFIT SHALL BE LEFT IN PLACE UNTIL THE TEMPORARY HOLD DOWN INSTALLATION IS COMPLETED.

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GENERATION 3 REINSTALLATION

GENERATION 3 RETROFIT TO REMAIN IN PLACE UNTIL THE TEMPORARY HOLD DOWN INSTALLATION IS COMPLETED.

DO NOT REUSE EXISTING A490 BOLTS REMOVED.

GENERATION 3 REINSTALL SHALL CONFORM TO DN 26658 INSTALLATION PROCEDURE DESCRIBED ON SHEET NO. S3.

REINSTALL WI8 SECTIONS AND TIGHTEN ANCHORS TO A PRELOAD OF 59 KIPS. AFTER TIGHTENING, VERIFY THAT EACH ANCHOR MAINTAINS ITS PRELOAD.

REINSTALL THE 6 INCH PLATE ACROSS THE BEARING IN ACCORDANCE WITH DN 26658, SHEET NO. S3, NOTES 9 THRU 13.

NEW 1" DIA. F3125 GRADE A490 HIGH STRENGTH BOLTS SHALL BE USED TO REINSTALL THE 6 INCH PLATE.

REINSTALL BOLTS BETWEEN THE TOP FLANGE OF THE W18 AND THE 6 INCH PLATE OVER THE BEARING. BOLTS SHALL BE EVENLY TENSIONED TO 64 KIPS.

LEGEND

GENERATION 3 RETROFIT REMOVAL LIMITS

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ND PLAN SET	ROUTE I-65 TEMP	JEFFE OI ORARY HOLI PREPARED BY	Y RSON CROSSING HIO RIVER D DOWN PL	IER 1L
ND PLAN SET	ROUTE 1–65 TEMP	DEFFE	Y RSON CROSSING HIO RIVER D DOWN PL D DOWN PL	IER 1L Sheet NO. S11
ND PLAN SET A	ROUTE I-65 TEMP	DEFARED BY CORARY HOLL PREPARED BY Chael Baker 1600 MBA	Y RSON CROSS ING HIO RIVER D DOWN PI Suffee, KY 40223 see: (602) 339-3557 KERINTL.COM	FR 1L SHEET NO. S11 DRAWING NO.

- PLATE P2. THE HOLE IN THE TOP LEG OF ANGLE

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PLAN SET	ROUTE I-65 HOLD	JEFFE OI DOWN PREPARED BY Baker	RSON CROSSING HO RIVER DETAILS – Lyndon Farm Court Lyndon Farm Court	1 Sheet NO. S13			
PLAN SET	ROUTE I-65 HOLD Michael	DOUNT JEFFE OI DOWN PREPARED BY Baker 1650 Louis Phonometer	RSON CROSSING HO RIVER DETAILS – Lyndon Farm Court wlie, KY 4023 sc: (502) 339-3557 sc: (502) 339-3557	1 Sheet NO. S13 Drawing NO.			

13. 2024 FILE NAME: A - SI5 - PIER IL PIN RE

USER: MaryJo.Dwyer

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SUGGESTED SEQUENCE OF CONSTRUCTION

- 1. REMOVE NUTS ON GENERATION 2 RETROFIT ANCHOR RODS.
- 2. INSTALL TEMPORARY HOLD DOWN TO BYPASS UPLIFT LOAD ON BEARING.
- 3. INSTALL JACKING SUPPORT UNDERNEATH FLOORBEAM TO BYPASS POSITIVE VERTICAL LOAD ON BEARING.
- 4. REMOVE EXISTING LOWER PIN AND ROTATE EXISTING LINK PLATE ASSEMBLY UP AND OUT OF THE WAY.
- 5. INSTALL NEW LINK PLATE ASSEMBLY ROTATED DOWN.
- 6. BORE NEW 10\% DIAMETER HOLE THROUGH EXISTING PLATES AND NEW LINK PLATES AT LOWER PIN LOCATION.
- 7. INSTALL NEW LOWER PIN.
- 8. REMOVE UPPER PIN AND EXISTING LINK PLATE ASSEMBLY.
- 9. ROTATE NEW LINK PLATE ASSEMBLY UP FOR TOP PIN HOLE BORING OPERATION.
- ¹⁷ 10. BORE NEW 10%6" DIAMETER HOLE THROUGH EXISTING PLATES AND NEW LINK PLATES AT UPPER PIN LOCATION.
 - 11. INSTALL NEW UPPER PIN.
 - 12. CLEAN AND PAINT EXISTING BEARING AND GENERATION 2 $\ensuremath{\mathsf{RETROFIT}}$
 - 13. REMOVE TEMPORARY HOLD DOWN AND JACKING SUPPORT.
 - 14. INSTALL NEW NUTS TO EXISTING GENERATION 2 RETROFIT ANCHOR RODS.

NOTES

- HOLES MARKED "RA" SHALL BE SUB-DRILLED 1/4" SMALLER THAN THE DIAMETER OF THE HOLE AS INDICATED AND LINE BORED TO SIZE WHILE ASSEMBLED WITH THE BEARING AND GUSSET.
- 2. FOR TEMPORARY HOLD DOWN, SEE SHEET NO. S11.
- 3. FOR NEW PIN DETAILS, SEE SHEET NO. S17.
- NEW LINK PLATE STEEL SHALL CONFORM TO ASTM A709 GRADE 50. PROVIDE CHARPY V-NOTCH IMPACT VALUES OF 25 FT-LBS AT 40°F.

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LINK PLATE DETAIL

SUGGESTED SEQUENCE OF CONSTRUCTION

- 1. REMOVE NUTS ON GENERATION 2 RETROFIT ANCHOR RODS.
- 2. INSTALL TEMPORARY HOLD DOWN TO BYPASS UPLIFT LOAD ON BEARING.
- 3. INSTALL JACKING SUPPORT UNDERNEATH FLOORBEAM TO BYPASS POSITIVE VERTICAL LOAD ON BEARING.
- 4. REMOVE LOWER PIN.
- 5. BORE NEW 10 $\%^{\circ}$ diameter hole through existing plates at lower PIN location.
- 6. INSTALL NEW LOWER PIN.
- 7. REMOVE UPPER PIN.
- 8. BORE NEW 10 $\%^{*}$ Diameter hole through existing plates at upper PIN location.
- 9. INSTALL NEW UPPER PIN.
- 10. CLEAN AND PAINT EXISTING BEARING AND GENERATION 2 RETROFIT.
- 11. REMOVE TEMPORARY HOLD DOWN AND JACKING SUPPORT.
- 12. INSTALL NEW NUTS TO EXISTING GENERATION 2 RETROFIT ANCHOR RODS.

LEGEND

* EX. GAP BETWEEN EX. DIAPH. PL AND EX. PIN = $\frac{1}{2}$ " NEW GAP BETWEEN EX. DIAPH. PL AND NEW PIN = $\frac{1}{4"}$

NOTES

- 1. FOR TEMPORARY HOLD DOWN, SEE SHEET NO. S11.
- 2. FOR SECTION C-C AND NEW PIN DETAILS, SEE SHEET NO. S17.

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PLAN SET	ROUTE I-65 PIER 6L	JEFF US BRG	CROSSING OHIO RIVER PIN REPLAC	EMENT
PLAN SET	ROUTE I-65 PIER 6L	JEFF US BRG PREPARED B	CROSSING OHIO RIVER PIN REPLACI	EMENT sheet no. \$16
PLAN SET A	ROUTE I-65 PIER 6L	JEFF US BRG PREPARED B Chael Baker E R N A T I O N A L	CROSSING OHIO RIVER PIN REPLAC	EMENT SHEET NO. ST6 DRAWING NO. 28935

SUGGESTED SEQUENCE OF CONSTRUCTION

- 1. REMOVE EXISTING GENERATION 2 RETROFIT. LEAVING ANCHOR RODS IN PLACE.
- 2. INSTALL TEMPORARY HOLD DOWN TO EXISTING GENERATION 2 RETROFIT ANCHOR RODS.
- 3. REMOVE EXISTING GENERATION 3 RETROFIT FOR CLEANING AND PAINTING, LEAVING ANCHOR RODS IN PLACE.
- 4. JACK UNDERNEATH FLOORBEAM TO RELIEVE VERTICAL LOAD ON BEARING.
- 5. REMOVE LOWER PIN.
- 6. BORE NEW 10% "DIAMETER HOLE THROUGH EXISTING PLATES AT LOWER PIN LOCATION.
- 7. INSTALL NEW LOWER PIN.
- 8. REMOVE UPPER PIN.
- 9. BORE NEW 10 $\%^{*}_{6}$ Diameter hole through existing plates at upper PIN location.
- 10. INSTALL NEW UPPER PIN.
- 11. PERFORM BEARING CONCRETE REPAIR.
- 12. CLEAN AND PAINT EXISTING BEARING.
- 13. REINSTALL GENERATION 3 RETROFIT.
- 14. REMOVE TEMPORARY HOLD DOWN.
- 15. WORK WITH OPTIONAL CONCEPT SHOWN ON SHEET NO. S18.

LEGEND

* EX. GAP BETWEEN EX. DIAPH. PL AND EX. PIN = $\frac{1}{2}$ " NEW GAP BETWEEN EX. DIAPH. PL AND NEW PIN = $\frac{1}{4}$ "

NOTES

1. FOR TEMPORARY HOLD DOWN, SEE SHEET NO. S12.

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	ROUTE	JEFF	CROSSING OHIO RIVER				
	ROUTE I-65 PIER 6L	JEFF DS BRG		CEMENT			
PLAN SET	ROUTE I–65 PIER 6L	JEFF DS BRG PREPARED	CROSSING OHIO RIVER PIN REPLAC	SHEET NO.			
PLAN SET	ROUTE I-65 PIER 6L	JEFF DS BRG PREPARED	CROSSING CROSSING OHIO RIVER PIN REPLAC BY 1650 Lyndon Farm Court Louisville, KY 40223	SHEET NO. SHEET NO.			
PLAN SET A	ROUTE I-65 PIER 6L	JEFF DS BRG PREPARED Chael Baker E R N A T I O N A L	CROSSING CROSSING OHIO RIVER PIN REPLACE BY 1650 Lyndon Farm Court Louiswille, KY 40223 MBAKERINTL.COM	SHEET NO. SHEET NO. S17 DRAWING NO. 28935			

CONCRETE REPAIR NOTES

THIS WORK CONSISTS OF REMOVING THE DETERIORATED CONCRETE ADJACENT TO AND BELOW THE BEARING MASONRY PLATE AND REPLACING IT WITH NON-SHRINK GROUT. THE MASONRY PLATE IS SURROUNDED BY HOLD DOWN SYSTEMS ON THE TRANSVERSE SIDES GENERATION 2) AND LONGITUDINAL SIDES (GENERATION 3). THE GENERATION 2 SYSTEM SHALL BE PERMANENTLY REMOVED; HOWEVER, THE ANCHOR RODS SHALL REMAIN IN PLACE TO BE USED WITH THE TEMPORARY HOLD DOWN SYSTEM. AFTER THE TEMPORARY HOLD DOWN IS IN PLACE, THE GENERATION 3 HOLD DOWN SHALL BE REMOVED FOR CLEANING AND PAINTING TO BE REPLACED AFTER THE CONCRETE REPAIR WORK IS COMPLETED.

THE CONTRACTOR SHALL SUBMIT A WRITTEN SEQUENCE OF THE SPECIFIC STEPS FOR THE CONCRETE REMOVAL TO THE ENGINEER PRIOR TO STARTING WORK. INCLUDE DETAILS OF ALL EQUIPMENT THAT WILL BE USED FOR THE CONCRETE REMOVAL, PAYING SPECIAL ATTENTION TO THE METHODS OF REMOVING THE CONCRETE BELOW THE MASONRY PLATE. CARE MUST BE TAKEN NOT TO DAMAGE THE EXISTING ANCHOR BOLTS.

ENSURE, IN THE PRESENCE OF THE ENGINEER, THAT ALL CONCRETE ADJACENT TO THE REMOVAL LIMITS IS SOUND. BASED ON RESULTS OF HAMMER SOUNDINGS, THE REMOVAL LIMITS MAY BE INCREASED AS DETERMINED BY THE ENGINEER.

SAW CUT THE CONCRETE REMOVAL BOUNDARIES 1 INCH DEEP MINIMUM.

USE PNEUMATIC HAMMERS AND CHISELS, NOT EXCEEDING 30 POUNDS, FOR REMOVAL.

REMOVE THE 3 INCH CONCRETE COVER EXPOSING THE PIER COLUMN TOP MAT #6 REINFORCING STEEL. ONCE INITIAL REMOVALS ARE MADE, PROCEED WITH UNDERCUTTING ALL OF THE TOP MAT EXPOSED BARS. PROVIDE 1 INCH MINIMUM CLEARANCE FOR UNDER BAR CLEANING AND FULL BAR CIRCUMFERENCE BONDING TO SURROUNDING CONCRETE. CONCRETE REMOVALS SHALL EXTEND ALONG THE BARS TO LOCATIONS WHERE THE BAR IS WELL BONDED TO SURROUNDING CONCRETE.

BLAST CLEAN ALL EXPOSED STEEL REINFORCEMENT TO REMOVE SCALE, RUST, GREASE, OIL AND OTHER MATERIAL THAT WOULD PREVENT ADHESION OF THE CONCRETE. CHECK THE CONCRETE AFTER CLEANING TO ENSURE THAT SURFACE IS FREE FROM ADDITIONAL LOOSE AGGREGATE AND TO ENSURE THAT ADDITIONAL DELAMINATION IS NOT PRESENT.

WITHIN 12 HOURS OF PLACING NON-SHRINK GROUT, BLAST CLEAN ALL SURFACES TO REMOVE DUST AND LOOSE MATERIAL WITH COMPRESSED AIR. COMPRESSED AIR MUST BE FREE FROM OIL AND WATER.

ENSURE THE SURFACE OF THE EXISTING CONCRETE IS IN A SATURATED SURFACE-DRY (SSD) CONDITION. REMOVE ALL FREE (PONDING) WATER JUST BEFORE PLACING THE GROUT. DO NOT USE AN EPOXY BOND COAT WITH THE GROUT.

PLACE NON-SHRINK GROUT AND CURE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS AND THE MANUFACTURER'S RECOMMENDATIONS. MATCH EXISTING TOP OF COLUMN SURFACE AND ENSURE THAT THE NEW SURFACE DRAINS AWAY FROM THE BEARING MASONRY PLATE.

LEGEND

APPROX. CONCRETE REMOVAL LIMITS

PIER COLUMN TOP MAT

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		REVISION							
	DATE: DEC	EMBER 2	024		CHECKE	D BY			
	DESIGNED E	BY:D BA	RON	М	BARON				
	DETAILED E	BY:MJ D	WYER	D	BARON				
	€o DE	Commonwealth of Kentucky DEPARTMENT OF HIGHWAYS							
			JEFF		SON				
	ROUTE			OHI	ROSSING O RIVER				
	PIER 6L	DS	BRG	СС	NCRETE	REPAIR			
PLAN SET			PREPARED [BY		SHEET NO.			
Α	— Mi	chael I	Baker	1650 Lyn Louisvill∉	idon Farm Court a, KY 40223	516			
				Phone: (MBAKEF	502) 339-3557 RINTL.COM	DRAWING NO.			
11		ERNAI	IUNAL			28935			

LEGEND

WIND LOCK REMOVAL LIMITS

NOTES

- 1. CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS BEFORE ORDERING ANY MATERIAL.
- 2. WIND LOCK BEAM ASSEMBLIES MAY ONLY BE REMOVED FOR REPLACEMENT WHILE THE PREDICTED SUSTAINED WIND SPEEDS FOR THE DURATION OF THE WORK ARE NOTE EXPECTED TO EXCEED 20 MPH.
- 3. REMOVE EXISTING WIND LOCK BEAM ASSEMBLIES AT PIER 6L AND REPLACE IN KIND. REMOVE AND REPLACE ONE WIND LOCK BEAM ASSEMBLY COMPLETELY BEFORE BEGINNING WORK ON THE SECOND WIND LOCK BEAM ASSEMBLY.
- 4. BEFORE INSTALLING NEW WIND LOCK BEAM ASSEMBLY, CLEAN AND PAINT EXISTING FLOORBEAM BOTTOM FLANGE FOR REUSE WITH THE NEW WIND LOCK IN ACCORDANCE WITH SECTION 614 OF THE STANDARD SPECIFICATIONS. MATCH THE NEW PAINT FINISH COAT COLOR WITH THE CURRENT COLOR OF THE STRUCTURE.
- 5. NEW WIND LOCK BEAM ASSEMBLIES SHALL RECEIVE A THREE COAT PAINT SYSTEM IN ACCORDANCE WITH SECTION 607.23.03 OF THE STANDARD SPECIFICATIONS.
- 6. THE COST TO CLEAN AND PAINT THE EXISTING FLOORBEAM BOTTOM FLANGE SHALL BE INCIDENTAL TO THE BID ITEM "STEEL REPAIR -WIND LOCK AT PIER 6L".
- 7. NEW STRUCTURAL STEEL SHALL BE ASTM A709 GRADE 36. CONTRACTOR HAS THE OPTION TO REPLACE ASTM A709 GRADE 36 STEEL WITH ASTM A709 GRADE 50 STEEL AT NO ADDITIONAL COST TO THE DEPARTMENT.
- 8. ALL BOLTS SHALL BE GALVANIZED 1" DIAMETER F3125 GRADE A325 HIGH STRENGTH BOLTS. GALVANIZE WASHERS PER ASTM F436.
- 9. FOR ADDITIONAL INFORMATION, SEE DESIGN DRAWING NO. 14744 AND SHOP DRAWINGS.
- 10. FIELD DRILL HOLES IN NEW W14×127 USING HOLES IN FLOORBEAM BOTTOM FLANGE AS A TEMPLATE.
- 11. WIND LOCK REPAIR TO BE COMPLETED BEFORE BEGINNING WORK ON BEARING PIN REPLACEMENT.
- 12. THIS WORK MAY BE CONDUCTED AND ACCESSED FROM THE PARKING LOT AREA BENEATH PIER 6L TO AVOID LANE CLOSURES ON I-65 SB, AS APPROVED BY THE ENGINEER.

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		REVISION		DATE
	DATE: DEC	EMBER 2024	CHECKED I	BY
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	DETAILED 6	BY: MJ DWYER	M BARON	
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		15		
		JEFFE	RSON	10
	ROUTE 1–65			10
	ROUTE I-65 W////	JEFFE or D LOCK REP	RSON RSON RIVER AIR AT PIE	R 6L
PLAN SET	ROUTE I-65 W/N	JEFFE OI D LOCK REP PREPARED BY	RSON RSON HIO RIVER AIR AT PIE	R 6L
PLAN SET	ROUTE I–65 W/W	JEFFE OI D LOCK REP PREPARED BY Chael Baker 1650	RSON CROSSING HIO RIVER AIR AT PIE	R 6L
plan set	ROUTE I-65 WINI	JEFFE OI D LOCK REP PREPARED BY Chael Baker 1650 MBA	Y RSON CROSSING HIO RIVER AIR AT PIE VIIII KY 40223 WIE (KY 4023) WIE (KY	R 6L Sheet NO. S19 Drawing NO.

$ \land $		
2	02-02-16	ADDL BRACING
$\overline{\mathbb{A}}$	12-29-15	REVISED DETAIL

		FOR INFORMATION		,						
		COUNTERWEIGHT DETAILS JFK BRIDGE REHABILITATION								
3	JK	Genesis Structures, Inc.	DRAWN BY	CHCK'D BY						
	BP	104. W. 9TH, SUITE 200 KANSAS CITY, MO. 64105	BP	DMR						
SIZE	JK	(P) 816-421-1520	10.1	0 15						
	DMR	PROJECT	SHEET NO.	Z-1J						
	BY	0593 - OHIO RIVER BRIDGE - DOWNTOWN	G50	-703						

Install steel shims to hold Bearing PL's in position. (not shown)

Installation Sequence Notes:

- 1. Lower "existing" PT bar to allow installation of additional structural elements.
- Continuity Member 3
- 2. Raise or remove "existing" Brg Plates Field drill all holes (using lug as template) from outside face of gusset. Install Shear Lug PLs (PL $1\frac{1}{2}x4\frac{3}{4}x2'-0$) in position on inside and PL Washer ($PL_8^3x4x2'-0$) on outside using only bolt (1)487 thru existing gusset. $\langle \overline{4} \rangle$ Field measure prior to fabrication. Fabricate
 - Continuity Members short by $\frac{1}{8}$ ". Provide $\frac{1}{16}$ " shims for tight fit.
 - Position Continuity Members and field drill $\langle 5. \rangle$ holes 2, 3, 5 & 6 thru holes in gusset and Shear Lug. Install and torque all bolts. Field weld Brg PL to Brg PL and to Brg PL to $\langle 6 \rangle$ Shear Lug PL as indicated.
- Outline of Brg PL above

Removal Sequence Notes: Option 1 - Leave in place

- 1. Remove PT bar and (2) $1\frac{3}{4}$ " Brg Plates.
- 2. Leave remaining members in place.
- All remaining members shall receive the full 3. contract specified paint system.
- All damaged paint shall be repaired. 4.

Option 2 - Full Removal

- Removal all temporary members, including 1. bearing plates, shear lugs and temporary bolts.
- 2. Fill holes with fully tensioned high strength bolts.
- Paint bolts with contract specified paint system. 3.
- 4. All damaged paint shall be repaired.

Material Notes:

- All bolts 1" Ø A325 with matching nuts and wahsers 1.
- 2. All material shall be Gr 50 u.n.o.

FOR INFORMATION ONLY COUNTERWEIGHT DETAILS

ĸ			JFK BRIDGE REHABILII	ATION			
		\wedge	GENESIS STRUCTURES, INC.	DRAWN BY	CHCK'D BY		
		/	104. W. 9TH, SUITE 200	JK	DMR		
			(P) 816-121-1520	DA	\TE		
			www.genesisstructures.com	01-2	1-16		
		PROJECT		SHEET NO.			
	ΒY	0593 - OHI	O RIVER BRIDGE-DOWNTOWN	G50	-705		

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FOR INFORMATION ONLY

TRANSPORTATION CABINET DEPARTMENT OF HIGHWAYS JEFFERSON COUNTY I-65 SOUTHBOUND OVER OHIO RIVER PLAN SET B – FINGER JOINT REPAIR

PROJECT

ESTIMATE OF QUANTITIES									
BID ITEM CODE	24430EC	24430EC	24430EC	24430EC					
BID ITEM	REM AND REPLACE FINGER EXPANSION JOINT - JOINT LO	REM AND REPLACE FINGER EXPANSION JOINT - JOINT L23	REM AND REPLACE FINGER EXPANSION JOINT - JOINT L23'	REM AND REPLACE FINGER EXPANSION JOINT - JOINT LO'					
UNIT	EACH	EACH	EACH	EACH					
BRIDGE TOTALS	1	1	1	1					

ITEM NO.

5-10074

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			SHEET		INDEX	
	SI	TITL	E SHEET			
	S2	GENE	ERAL NOTES			
	S3	BRIC	DGE PLAN AND ELE	۷A	TION	
	S4	JOIN	NT LO REMOVAL DE	TA	ILS	
	S5	JOIN	NT L23 REMOVAL D	ETA	AILS	
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	S8-S9	REM	OVAL SECTIONS			
	S10-S11	JOIN	NT LO RECONSTRUC	TIC	N	
	S12-S13	JOIN	NT L23 RECONSTRU	CTI	ION	
	S14-S15	JOIN	NT L23' RECONSTRU	CTI	ION	
	S16-S17	JOIN	IT LO' RECONSTRUC	TIC	N	
	S18-S19	REC	ONSTRUCTION SECT	101	VS	
	S20	DIAF	PHRAGM DETAILS			
	S21	BILL	OF REINFORCEMEN	NT		
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SPECIFICATIONS

ALL REFERENCES TO THE STANDARD SPECIFICATIONS ARE TO THE CURRENT EDITION OF THE KENTUCKY DEPARTMENT OF HIGHWAYS STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, AND CURRENT SUPPLEMENTAL SPECIFICATIONS. ALL REFERENCES TO THE AASHTO SPECIFICATIONS ARE TO THE FOURTH EDITION 2017 AASHTO LRFD BRIDGE CONSTRUCTION SPECIFICATION AND 2020 AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, NINTH EDITION.

COMPLETION OF THE STRUCTURE

THE CONTRACTOR IS REQUIRED TO COMPLETE THE STRUCTURE IN ACCORDANCE WITH THE PLANS AND SPECIFICATIONS. MATERIAL, LABOR OR CONSTRUCTION OPERATIONS, NOT OTHERWISE SPECIFIED, ARE TO BE INCLUDED IN THE APPROPRIATE BID ITEMS. THIS MAY INCLUDE REMOVAL OF ALL, OR PARTS, OF EXISTING STRUCTURES, PHASE CONSTRUCTION, INCIDENTAL MATERIALS, LABOR OR ANYTHING ELSE REQUIRED TO COMPLETE THE STRUCTURE.

ON-SITE INSPECTION

THE CONTRACTOR IS RESPONSIBLE FOR MAKNG A SITE INSPECTION TO BECOME FAMILIAR WITH THE WORK TO BE DONE AND TO MAKE APPROPRIATE ALLOWANCES FOR ALL WORK INCLUDED IN THE APPROPRIATE BID ITEMS. A SUITABLE METHOD OF PERFORMING THE WORK DESCRIBED HEREIN SHOULD BE INVESTIGATED. SUBMISSION OF A BID WILL BE CONSIDERED EVIDENCE OF THIS INVESTIGATION HAVING BEEN MADE. THE CONTRACTOR WILL NOT BE PAID EXTRA BECAUSE OF SITE CONDITIONS.

MAINTENANCE OF TRAFFIC

TRAFFIC SHALL BE MAINTAINED AT ALL TIMES IN ACCORDANCE WITH THE PLANS AND SPECIAL NOTES.

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USE CLASS AA (4,000 PSI) IN ACCORDANCE WITH SECTION 601 OF THE STANDARD SPECIFICATIONS.

WELDING SPECIFICATIONS

ALL WELDING AND WELDING MATERIALS, EXCEPT FOR REINFORCEMENT, SHALL CONFORM TO JOINT SPECIFICATIONS ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE. NON-DESTRUCTIVE TESTING BY THE CONTRACTOR WILL NOT BE REQUIRED. PAYMENT FOR WELDING, WELDING MATERIALS, STRAIGHTENING, ALTERING AND BURNING NEW OR EXISTING STEEL SHALL BE INCIDENTAL TO THE APPROPRIATE PAY ITEMS.

STUD WELDING

SHEAR STUDS SHALL BE WELDED IN ACCORDANCE WITH AWS SPECIFICATIONS.

DIMENSIONS

DIMENSIONS SHOWN ON THE PLANS ARE TAKEN FROM THE ORIGINAL CONTRACT PLANS, SUBSEQUENT RECONSTRUCTION AND SHOP DRAWING PLANS. THE CONTRACTOR SHALL VERIFY DIMENSIONS, INCLUDING THICKNESSES OF PARTS, WITH FIELD MEASUREMENTS PRIOR TO ORDERING MATERIALS OR FABRICATING STEEL. ALL PLAN DIMENSIONS ARE FOR A NORMAL TEMPERATURE OF 60 DEG F. LAYOUT DIMENSIONS ARE HORIZONTAL MEASUREMENTS AND DO NOT NECESSARILY REFLECT REVISIONS.

PLANS OF EXISTING STRUCTURE

PLANS AND SHOP DRAWINGS OF THE EXISTING STRUCTURE ARE AVAILABLE AS AN AID TO THE CONTRACTOR AND SHALL BE USED TO SUPPLEMENT DETAILS NOT SHOWN ON THE PLANS. THE COMPLETENESS OF THESE DRAWINGS IS NOT GUARANTEED AND NO RESPONSIBILITY IS ASSUMED BY KYTC FOR THEIR ACCURACY. AS-BUILT PLANS AND FINGER JOINT DRAWINGS INCLUDE:

I-65 SB AS-BUIL	T ORBP	056B0002141L REHAB PLANS & BU 2-232
I-65 SB AS-BUIL	T ORBP	BRIDGE NO. A027-2
I-65 SB INDIANA	APPROACH	BUILDABLE UNIT NUMBER 2-200
ORBP AS-BUILT		INTELLIGENT TRANSPORTATION SYSTEMS IT8133
I-65 SB FINGER	JOINT REPAIR	DN 28096
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JOINT L23		D-32797
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EXISTING STEEL REINFORCEMENT

PAYMENT FOR CUTTING, BENDING, SPLICING AND CLEANING EXISTING REINFORCING BARS SHALL BE INCLUDED IN THE APPROPRIATE BID ITEM.

SAWCUTTING

PRIOR TO THE REMOVAL OF THE EXISTING CONCRETE MASONRY, CUT THE SURFACE WITH A CONCRETE SAW TO THE DEPTH NOTED ON THE PLANS OR ONE INCH TO FACILITATE A NEAT LINE. PAYMENT FOR CUTTING CONCRETE SHALL BE INCIDENTAL TO THE APPROPRIATE PAY ITEM.

CONCRETE REMOVAL

PERFORM WORK CAREFULLY DURING SLAB REMOVAL TO PROTECT PORTIONS OF SUCH SYSTEMS THAT ARE TO BE SALVAGED AND INCORPORATED INTO THE PROPOSED STRUCTURE. ALL REMOVAL SHALL BE TO NEAT SAW CUT LINES. FEATHER EDGES WILL NOT BE PERMITTED. SAW CUT BOUNDARIES OF PROPOSED CONCRETE REMOVAL 1 INCH DEEP.

THE CONTRACTOR MAY REMOVE CONCRETE BY CUTTING AND BY MEANS OF HAND OPERATED PNEUMATIC HAMMERS EMPLOYING POINTED OR BLUNTED CHISEL TYPE TOOLS. DO NOT PLACE PNEUMATIC HAMMERS IN DIRECT CONTACT WITH REINFORCING STEEL THAT IS TO BE RETAINED IN THE PROPOSED STRUCTURE. REMOVE CONCRETE TO THE SURFACE OF THE STEEL STAY-IN-PLACE FORMS. LEAVE EXISTING REINFORCING STEEL IN PLACE AS SHOWN ON THE PLANS.

PRIOR TO CONCRETE PLACEMENT, ABRASIVELY CLEAN JOINT SURFACES AND EXISTING PRIOR TO CONCRETE PLACEMENT, ABRASIVELY CLEAN JOINT SURFACES AND EXISTING EXPOSED REINFORCEMENT TO REMOVE LOOSE AND DISINTEGRATED CONCRETE AND LOOSE RUST. THOROUGHLY CLEAN THE JOINT SURFACE AND EXPOSED REINFORCEMENT OF ALL DIRT, DUST, RUST OR OTHER FOREIGN MATERIAL BY THE USE OF WATER, AIR PRESSURE OR OTHER METHODS THAT PRODUCE SATISFACTORY RESULTS. EXISTING REINFORCING STEEL DOES NOT HAVE TO HAVE A BRIGHT STEEL FINISH, HOWEVER, REMOVE ALL PACK AND LOOSE RUST. THOROUGHLY DRENCH EXISTING CONCRETE SURFACES WITH CLEAN WATER AND AND AND AND AND AND AND AND ADDRENCE AND ADDRENCE AND ADDRENCE AND ADDRENCE AND ADDRENCE AND ADDRENCE WATER AND ALLOW TO DRY TO A DAMP CONDITION BEFORE PLACING CONCRETE.

REINFORCING BARS WHICH ARE SHOWN ON THE PLANS AS REMAINING AND WHICH ARE DAMAGED BY THE CONTRACTOR'S OPERATIONS SHALL BE REPLACED WITH NEW EPOXY COATED BARS OF THE SAME SIZE AND SHAPE, AS APPROVED BY THE ENGINEER. NO ADDITIONAL PAYMENT WILL BE MADE FOR THOSE BARS.

STAY-IN-PLACE FORMS WHICH ARE SHOWN ON THE PLANS AS REMAINING AND WHICH ARE DAMAGED BY THE CONTRACTOR'S OPERATIONS SHALL BE REPLACED WITH NEW STAY-IN-PLACE FORMS OF THE SAME SIZE AND SHAPE, AS APPROVED BY THE ENGINEER. NO ADDITIONAL PAYMENT WILL BE MADE FOR THOSE FORMS.

PAYMENT FOR THIS WORK SHALL BE INCLUDED IN THE APPROPRIATE BID ITEM.

REMOVE STEEL

ALL EXISTING STEEL THAT IS REMOVED AND NOT REUSED IN THE COMPLETED STRUCTURE SHALL BECOME THE PROPERTY OF THE CONTRACTOR AND SHALL BE REMOVED FROM THE BRIDGE SITE.

BONDING NEW CONCRETE TO PREVIOUSLY PLACED CONCRETE

WHERE A BONDED CONSTRUCTION JOINT IS CALLED FOR ON THE PLANS, NEW CONCRETE SHALL BE BONDED TO PREVIOUSLY PLACED (CURED) CONCRETE WITH A TWO-COMPONENT EPOXY RESIN SYSTEM CONFORMING TO SECTIONS 511 AND 826 OF THE SPECIFICATIONS. PAYMENT FOR THIS WORK, INCLUDING ALL LABOR, TOOLS AND MATERIALS SHALL BE INCIDENTAL TO THE APPROPRIATE BID ITEM.

EXISTING EXPANSION JOINT FINGER PLATE SYSTEM

THE EXISTING FINGER PLATE ASSEMBLY COMPRISES SEGMENTED FINGER PLATES FASTENED WITH HIGH STRENGTH BOLTS TO COUPLING NUTS WELDED TO EMBEDDED PLATES ANCHORED IN CONCRETE. NEOPRENE DRAINAGE TROUGHS INSTALLED WITH THE FINGER JOINTS CATCH AND CHANNEL AWAY WATER AND DEBRIS. SHOP DRAWINGS OF THE EXISTING FINGER JOINTS SHALL BE USED AS A SUPPLEMENT FOR DETAILS NOT SHOWN ON THE PLANS.

UTILITIES

BEFORE BEGINNING WORK, LOCATE ALL EXISTING UTILITIES. CONSIDER LOCATION OF UTILITIES SHOWN ON THE DRAWINGS TO BE APPROXIMATE AND FOR INFORMATIONAL PURPOSES ONLY. THE DEPARTMENT DOES NOT WARRANT THE LOCATIONS AND ASSUMES NO RESPONSIBILITY FOR THE ACCURACY OR COMPLETENESS. THE CONTRACTOR MUST MAKE THEIR OWN DETERMINATION. EXCEPT AS SHOWN ON THE PLANS, WORK AROUND AND DO NOT DISTURB EXISTING UTILITIES. UTILITIES DAMAGED BY THE CONTRACTOR'S DEFENDION SHOWN ON THE DEFENDENCE AS ADDROVED BY THE CONTRACTOR'S OPERATION SHALL BE REPLACED, AS APPROVED BY THE ENGINEER. NO ADDITIONAL PAYMENT WILL BE MADE.

PLAN SET A AND PLAN SET B COORDINATION

WORK ON THE TRUSS BEARINGS AND FINGER JOINT REPLACEMENT CAN BE STAGED CONCURRENTLY BY THE CONTRACTOR IF DESIRED, AS LONG AS THE FINGER JOINT REPLACEMENT RESULTS IN PROPER PHYSICAL ALIGNMENT AND FINAL ELEVATIONS. IF THE FINAL RESULTS DO NOT COMPLY WITH THIS REQUIREMENT, CORRECTIONS MUST BE MADE AS APPROPRIATE AT THE CONTRACTOR'S EXPENSE.

DAMAGE TO THE STRUCTURE

THE CONTRACTOR SHALL BEAR FULL RESPONSIBILITY AND EXPENSE FOR ANY AND ALL DAMAGE TO THE STRUCTURE, INCLUDING TRUSS MEMBERS, DURING THE REPAIR AND RETROFIT WORK; EVEN TO THE REMOVAL AND REPLACEMENT OF TRUSS MEMBERS AND FALLEN SPANS, SHOULD THE DAMAGE RESULT FROM THE CONTRACTOR'S ACTIONS.

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THE WORK FOR EACH OF THESE ITEMS CONSISTS OF REMOVAL AND REPLACEMENT OF THE EXISTING FINGER JOINT AND DRAINAGE TROUGH. IT INCLUDES ANCHORING THE NEW FINGER PLATE TO NEW OR EXISTING DIAPHRAGMS FRAMED BETWEEN EXISTING STRINGERS, AND DESIGNING THE NEW DRAIN TROUGH TO MAKE USE OF THE EXISTING DRAINAGE COLLECTOR SYSTEM. SEE SPECIAL NOTE FOR FINGER EXPANSION JOINTS.

WORK.

CONTRACTOR'S SUBMITTALS

WHERE REQUIRED BY THE PLANS AND SPECIFICATIONS, THE CONTRACTOR SHALL SUBMIT DESCRIPTIVE INFORMATION THAT WILL ENABLE THE ENGINEER TO DETERMINE WHETHER THE CONTRACTOR'S PROPOSED MATERIALS, EQUIPMENT, AND WORK METHODS ARE IN GENERAL CONFORMANCE WITH THE PLANS AND SPECIFICATIONS.

THE CONTRACTOR SHALL SUBMIT SEQUENCES, TECHNIQUES AND PROCEDURES OF CONSTRUCTION INCLUDING, BUT NOT LIMITED TO, LABOR, MATERIALS, TEMPORARY STRUCTURES, TOOLS, CONSTRUCTION EQUIPMENT, AND ALL INCIDENTAL OR TEMPORARY DEVICES REQUIRED TO ACCOMPLISH THE RESULT INTENDED BY THIS CONTRACT.

MECHANICAL COUPLERS

PROVIDE MECHANICAL BUTT SPLICE SLEEVE COUPLERS USING A SERIES OF CONE-POINTED HEX-HEAD SHEAR SCREWS ARRANGED ALONG THE LONGITUDINAL AXIS EMBEDDED INTO THE BAR ENDS WITHIN THE CONFINED INTERIOR OF THE COUPLER. IN THE CASE OF BUTT SPLICES, INSERT REINFORCING BARS FROM EACH END TO A CENTER STOP.

PROVIDE MECHANICAL COUPLERS IN ACCORDANCE WITH THE REQUIREMENTS OF 602.03.06 OF THE STANDARD SPECIFICATIONS. IN ACCORDANCE WITH THE SPECIFICATIONS, SUBMIT SPECIMENS OF EACH BAR SIZE SPLICE TO THE DIVISION OF MATERIALS FOR TESTING BEFORE INCORPORATING THE SPLICES INTO THE WORK. ENSURE SAMPLES ARE MADE BY THE SAME PERSONNEL USING EQUIPMENT TO BE USED TO MAKE THE PRODUCTION SPLICES.

INSTALL MECHANICAL SPLICE COUPLING SLEEVES IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDED PROCEDURES. SUBMIT THE INSTALLATION PROCEDURE, INCLUDING MANUFACTURER APPROVALS, TO THE ENGINEER FOR REVIEW BEFORE BEGINNING SPLICING.

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THE BID ITEM FOR EACH OF THESE ITEMS SHALL BE FULL PAYMENT FOR ALL MATERIAL, TOOLS, EQUIPMENT, LABOR, ACCESS AND INCIDENTALS, INCLUDING BUT NOT LIMITED TO STRUCTURAL STEEL, CONCRETE AND REBAR, TO COMPLETE THE

IF USING EPOXY COATED REINFORCEMENT, PROVIDE EPOXY COATED MECHANICAL SPLICES IN ACCORDANCE WITH AASHTO M284. COVER THE UNCOATED SURFACE OF THE SHEARED OFF BOLTS WITH EPOXY PREPARED FROM AN APPROVED EPOXY TOUCH UP KIT. ENSURE EPOXY COATED SPLICES AFTER INSTALLATION.

MECHANICAL BUTT SPLICE COUPLERS MAY BE:

BARSPLICE PRODUCTS, INC. - 4900 WEBSTER STREET - DAYTON, OHIO 45414 DAYTON SUPERIOR - 1125 BYERS ROAD - MIAMISBURG, OHIO 45342 nVENT LENTON - 34600 SOLON ROAD - SOLON, OHIO 44139

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STANDARD SPECIFICATIONS WITH FINGER PLATES IN PLACE TO MATCH EXISTING PROFILE.

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STANDARD SPECIFICATIONS WITH FINGER PLATES IN PLACE TO MATCH EXISTING PROFILE.

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NOTES

- 1. SEE GENERAL NOTES FOR PLANS OF EXISTING STRUCTURE AND WORK ITEMS.
- 2. LEAVE BARRIER REINFORCEMENT THAT EXTENDS INTO THE SLAB AND TRANSVERSE OVERHANG REINFORCEMENT IN PLACE. REMOVE CONCRETE WITH A METHOD THAT WILL NOT DAMAGE EXISTING REINFORCEMENT TO REMAIN IN THE STRUCTURE.
- 3. EXISTING LONGITUDINAL REBAR WITHIN THE LIMITS OF THE CONSTRUCTION SHALL BE REMOVED AND REPLACED WITH NEW EPOXY COATED BARS OF THE SAME SIZE AND SHAPE AS SHOWN ON THE PLANS.
- 4. EXISTING TRANSVERSE REBAR WITHIN THE LIMITS OF THE CONSTRUCTION AS SHOWN ON THE PLANS SHALL BE CUTOFF 3'-10" MINIMUM FROM THE SLAB REMOVAL LINE. CONCRETE SHALL BE REMOVED TO EXPOSE A 3'-10" MINIMUM DOWEL TO LAP WITH NEW REBAR. BLAST CLEAN THE EXPOSED REINFORCEMENT.
- 5. SUPPORT EXISTING PARAPET AS NEEDED. COORDINATE PARAPET WORK ON PLAN SET C -TRUSS REPAIR AT L23 WITH THIS REMOVAL.
- 6. SEE SHEET NOS. S4-S7 FOR LOCATIONS OF SECTIONS E-E AND F-F.

REVISION ΠΔΤΕ DATE: DECEMBER 2024 CHECKED BY DESIGNED BY: M BARON J STITH DETAILED BY: MJ DWYER D BARON Commonwealth of Kentucky DEPARTMENT OF HIGHWAYS **JEFFERSON** ROUT I-65 **OHIO RIVER REMOVAL SECTIONS** PREPARED F SHEET NO PLAN SET Michael Baker http://www.communication.com/ http://wwww.communication.com/ http://wwwwwwww.communication.com/ http://www S9 RAWING В 28935

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NOTES

- 1. SEE GENERAL NOTES FOR BRIDGE ITEMS AND PLANS OF EXISTING STRUCTURE.
- 2. CONTRACTOR SHALL VERIFY DIMENSIONS NECESSARY FOR THE PROPER FIT OF STEEL PIECES PRIOR TO THE FABRICATION OF THE STEEL.
- 3. CONSTRUCT EXPANSION JOINT TO MATCH ROADWAY GRADE AND CROSS SLOPE.
- 3. STRUCTURAL STEEL SHALL CONFORM TO AASHTO M270, GRADE 50, U.N.O.
- 4. BOLTS SHALL BE $\%^{\rm s}$ DIA. F3125 grade A325 type 1 galvanized high strength bolts.
- 5. NEW HOLES IN EXISTING MATERIAL SHALL BE DRILLED USING A TEMPLATE.
- 6. STRUCTURAL STEEL AND HARDWARE SHALL BE GALVANIZED U.N.O.
- 7. KNOCK OFF TRACTION STUDS SHALL BE ANTI-SKID TYPE 5/16" NOMINAL DIAMETER BY 1/4" HEIGHT. ALTERNATE PATTERNS OTHER THAN SHOWN MUST BE APPROVED BY THE ENGINEER.
- 8. PLACE CONCRETE UNDER FINGER JOINTS AND VIBRATE UNTIL THE CONCRETE IS FORCED THROUGH THE 7/8" DIAMETER AIR HOLES. STRIKE OFF EXCESS CONCRETE. AFTER CONCRETE HAS CURED, INSPECT THE HOLES AND REMOVE UNSOUND CONCRETE. CLEAN THE HOLES WITH AN AIR JET AND FILL WITH APPROVED SEALER.
- 9. BEFORE PLACING BLOCKOUT CONCRETE, APPLY APPROVED EPOXY BONDING AGENT TO ALL DECK CONSTRUCTION JOINTS.
- 10. PLACE CLASS AA CONCRETE IN THE BLOCKOUT EXCEPT AS SPECIFIED OR INDICATED.
- 11. TACK WELD AFTER CONFIRMATION OF THE FINGER JOINT INSTALLATION BY THE ENGINEER.
- 12. FOR DRAINAGE TROUGH, SEE SPECIAL NOTE FOR FINGER EXPANSION JOINT.

FOR EXISTING TROUGH, SEE WABO FINGER EXPANSION JOINT DETAILS SHOP DRAWING NO. D-32819.

FOR LOCATIONS OF DRAINAGE DOWNSPOUTS AND DRAINAGE COLLECTOR SYSTEM, SEE AS-BUILT SLAB REINFORCING PLANS SHEETS 2S7032 THRU 2S7037 AND AS-BUILT SLAB DRAINAGE SHEETS 2S7045 THRU 2S7047.

13. FIELD WELD 5/8" PLATES AT CROWN BEFORE PLACING PHASE 2 CONCRETE.

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NOTES

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- 9. BEFORE PLACING BLOCKOUT CONCRETE, APPLY APPROVED EPOXY BONDING AGENT TO ALL DECK CONSTRUCTION JOINTS.
- 10. PLACE CLASS AA CONCRETE IN THE BLOCKOUT EXCEPT AS SPECIFIED OR INDICATED.
- 11. TACK WELD AFTER CONFIRMATION OF THE FINGER JOINT INSTALLATION BY THE ENGINEER.
- 12. FOR DRAINAGE TROUGH, SEE SPECIAL NOTE FOR FINGER EXPANSION JOINT.

FOR EXISTING TROUGH, SEE WABO FINGER EXPANSION JOINT DETAILS SHOP DRAWING NO. D-32797.

FOR LOCATIONS OF DRAINAGE DOWNSPOUTS AND DRAINAGE COLLECTOR SYSTEM, SEE AS-BUILT SLAB REINFORCING PLANS SHEETS 2S7032 THRU 2S7037 AND AS-BUILT SLAB DRAINAGE SHEETS 2S7045 THRU 2S7047.

13. FIELD WELD 5/8" PLATES AT CROWN BEFORE PLACING PHASE 2 CONCRETE.

		REVISION		DATE						
	DATE: DECE	EMBER 2024	CHECKED E	3Y						
	DESIGNED E	BY:M BARON	J STITH							
	DETAILED E	BY: MJ DWYER	D BARON							
	€o DE	mmonwealth PARTMENT (of Kentuck DF HIGHWA	.y YS						
		JEFFE	RSON							
	ROUTE	01	CROSSING							
	JOII	VT L23 RECO	ONSTRUCTI	ON						
PLAN SET		PREPARED BY		SHEET NO.						
	Mi	chael Baker	Lyndon Farm Court ville, KY 40223	S13						
В										
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SECTION A-A

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TOM)				
		REVISION		DATE
с. С	DATE: DEC	EMBER 2024	CHECKED	BY
0	DESIGNED B	BY:M BARON	J STITH	
	DETAILED E	BY: MJ DWYER	D BARON	
PLATE	€o DE	ommonwealth PARTMENT	1 of Kentuck OF HIGHWA	sy YS
		JEFFE	RSON	
	ROUTE	(CROSSING DHIO RIVER	
	JOI	NT L23' RE	CONSTRUCT	ION
N SET		PREPARED BY		SHEET NO.
	Mi	chael Baker	50 Lyndon Farm Court Julsville, KY 40223	S14
В	I N T		none: (502) 339-3557 BAKERINTL.COM	DRAWING NO. 28935
	<u>B</u>			

NOTES

- 1. SEE GENERAL NOTES FOR BRIDGE ITEMS AND PLANS OF EXISTING STRUCTURE.
- 2. CONTRACTOR SHALL VERIFY DIMENSIONS NECESSARY FOR THE PROPER FIT OF STEEL PIECES PRIOR TO THE FABRICATION OF THE STEEL.
- 3. CONSTRUCT EXPANSION JOINT TO MATCH ROADWAY GRADE AND CROSS SLOPE.
- 3. STRUCTURAL STEEL SHALL CONFORM TO AASHTO M270, GRADE 50, U.N.O.
- 4. BOLTS SHALL BE % DIA. F3125 GRADE A325 TYPE 1 GALVANIZED HIGH STRENGTH BOLTS.
- 5. NEW HOLES IN EXISTING MATERIAL SHALL BE DRILLED USING A TEMPLATE.
- 6. STRUCTURAL STEEL AND HARDWARE SHALL BE GALVANIZED U.N.O.
- 7. KNOCK OFF TRACTION STUDS SHALL BE ANTI-SKID TYPE 5/16" NOMINAL DIAMETER BY 1/4" HEIGHT. ALTERNATE PATTERNS OTHER THAN SHOWN MUST BE APPROVED BY THE ENGINEER.
- 8. PLACE CONCRETE UNDER FINGER JOINTS AND VIBRATE UNTIL THE CONCRETE IS FORCED THROUGH THE 7/8" DIAMETER AIR HOLES. STRIKE OFF EXCESS CONCRETE. AFTER CONCRETE HAS CURED, INSPECT THE HOLES AND REMOVE UNSOUND CONCRETE. CLEAN THE HOLES WITH AN AIR JET AND FILL WITH APPROVED SEALER.
- BEFORE PLACING BLOCKOUT CONCRETE, APPLY APPROVED EPOXY BONDING AGENT TO ALL DECK CONSTRUCTION JOINTS.
- 10. PLACE CLASS AA CONCRETE IN THE BLOCKOUT EXCEPT AS SPECIFIED OR INDICATED.
- 11. TACK WELD AFTER CONFIRMATION OF THE FINGER JOINT INSTALLATION BY THE ENGINEER.
- 12. FOR DRAINAGE TROUGH, SEE SPECIAL NOTE FOR FINGER EXPANSION JOINT.

FOR EXISTING TROUGH, SEE WABO FINGER EXPANSION JOINT DETAILS SHOP DRAWING NO. D-32834.

FOR LOCATIONS OF DRAINAGE DOWNSPOUTS AND DRAINAGE COLLECTOR SYSTEM, SEE AS-BUILT SLAB REINFORCING PLANS SHEETS 2S7032 THRU 2S7037 AND AS-BUILT SLAB DRAINAGE SHEETS 2S7045 THRU 2S7047.

13. FIELD WELD 5/8" PLATES AT CROWN BEFORE PLACING PHASE 2 CONCRETE.

AT THE				
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ANGLE		REVISION		DATE
	DATE: DEC	EMBER 2024	CHECKED E	3Y
	DESIGNED B	BY:M BARON	J STITH	
	DETAILED 8	BY: MJ DWYER	D BARON	
EDED PREVENT	€o De	mmonwealth PARTMENT (of Kentuck OF HIGHWAY	y YS
) W/		JEFFE	RSON	
	ROUTE	0	CROSSING	
	JOI	NT L23' REC	CONSTRUCT	ION
PLAN SET		PREPARED BY		SHEET NO.
В	Mi	Chael Baker ERNATIONAL	I Lyndon Farm Court sville, KY 40223 iec (502) 339-3557 KERINTL.COM	DRAWING NO. 28935

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28935

NOTES

- 1. SEE GENERAL NOTES FOR BRIDGE ITEMS AND PLANS OF EXISTING STRUCTURE.
- 2. CONTRACTOR SHALL VERIFY DIMENSIONS NECESSARY FOR THE PROPER FIT OF STEEL PIECES PRIOR TO THE FABRICATION OF THE STEEL.
- 3. CONSTRUCT EXPANSION JOINT TO MATCH ROADWAY GRADE AND CROSS SLOPE.
- 3. STRUCTURAL STEEL SHALL CONFORM TO AASHTO M270, GRADE 50, U.N.O.
- 4. BOLTS SHALL BE $\%^{\rm s}$ DIA. F3125 grade A325 type 1 galvanized high strength bolts.
- 5. NEW HOLES IN EXISTING MATERIAL SHALL BE DRILLED USING A TEMPLATE.
- 6. STRUCTURAL STEEL AND HARDWARE SHALL BE GALVANIZED U.N.O.
- 7. KNOCK OFF TRACTION STUDS SHALL BE ANTI-SKID TYPE 5/16" NOMINAL DIAMETER BY 1/4" HEIGHT. ALTERNATE PATTERNS OTHER THAN SHOWN MUST BE APPROVED BY THE ENGINEER.
- 8. PLACE CONCRETE UNDER FINGER JOINTS AND VIBRATE UNTIL THE CONCRETE IS FORCED THROUGH THE 7/8" DIAMETER AIR HOLES. STRIKE OFF EXCESS CONCRETE. AFTER CONCRETE HAS CURED, INSPECT THE HOLES AND REMOVE UNSOUND CONCRETE. CLEAN THE HOLES WITH AN AIR JET AND FILL WITH APPROVED SEALER.
- BEFORE PLACING BLOCKOUT CONCRETE, APPLY APPROVED EPOXY BONDING AGENT TO ALL DECK CONSTRUCTION JOINTS.
- 10. PLACE CLASS AA CONCRETE IN THE BLOCKOUT EXCEPT AS SPECIFIED OR INDICATED.
- 11. TACK WELD AFTER CONFIRMATION OF THE FINGER JOINT INSTALLATION BY THE ENGINEER.
- 12. FOR DRAINAGE TROUGH, SEE SPECIAL NOTE FOR FINGER EXPANSION JOINT.

FOR EXISTING TROUGH, SEE WABO FINGER EXPANSION JOINT DETAILS SHOP DRAWING NO. D-32798.

FOR LOCATIONS OF DRAINAGE DOWNSPOUTS AND DRAINAGE COLLECTOR SYSTEM, SEE AS-BUILT SLAB REINFORCING PLANS SHEETS 2S7032 THRU 2S7037 AND AS-BUILT SLAB DRAINAGE SHEETS 2S7045 THRU 2S7047.

13. FIELD WELD 5/8" PLATES AT CROWN BEFORE PLACING PHASE 2 CONCRETE.

		REVISION		DATE
	DATE: DECE	EMBER 2024	CHECKED 6	ЗY
	DESIGNED E	BY: M BARON	J STITH	
	DETAILED E	BY: MJ DWYER	D BARON	
	Сo	mmonwealth	of Kentuck	4
	DE	PARTMENT (OF HIGHWAY	vs I
		COUNT	Y	
		JEFFE	RSON	
	ROUTE		CROSSING	
	-65	0	HIO RIVER	
	JOI	NT LO' RECO	ONSTRUCTIO	ON N
PLAN SET		PREPARED BY		SHEET NO.
	Miz		Lyndon Farm Court	S17
В	IVII		Ne (502) 339-3557	DRAWING NO.
U		ERNATIONAL MBA		28935

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NOTES

- 1. STRUCTURAL STEEL SHALL CONFORM TO AASHTO M270, GRADE 50.
- BOLTS SHALL BE 7/8" DIA. F3125 GRADE A325 TYPE I GALVANIZED HIGH STRENGTH BOLTS.
- 3. STRUCTURAL STEEL SHALL BE PREPARED AND PAINTED WITH A THREE COAT SYSTEM FOR THE KYTC APPROVED MATERIAL LIST.

	BILL OF REINFORCEMENT - L0																	
MARK	TYDE	NO	917E	LEN	IGTH			4	E	3	(C D E T IN FT IN FT IN F D 11 0 7 0 7 3/4 D 11 0 7 0 7 3/4		F				
WARK	TTFE	NO.	SIZE	FT	IN	LOCATION	FT	IN	FT	IN	FT	IN	FT	IN	FT	IN	FT	IN
4S01E (*)	STR	91	#4	3	5	LONGITUDINAL TOP												
							_	-										
5S01E (*)	STR	185	#5	3	10	LONGITUDINAL BOTTOM												
5S02E	23s	86	#5	3	6	THICKENED SLAB	0	10	1	2	0	11	0	7	0	7 3/4	0	7 3/4
5S03E (*)	STR	62	#5	2	6	LONGITUDINAL TOP												
6S01E (*)	STR	90	#6	3	8	LONGITUDINAL BOTTOM												
6S02E	STR	30	#6	7	11	TRANSVERSE BETWEEN STRINGERS												
6S03E (*)	STR	26	#6	46	3	TRANSVERSE TOP & BOTTOM												
6S04E	STR	40	#6	2	4	TRANSVERSE BETWEEN WTs												
6S05E	STR	20	#6	2	5	TRANSVERSE OVER STRINGERS												
6S06E	STR	3	#6	3	11	TRANSVERSE BETWEEN STRINGERS												
6S07E	STR	26	#6	45	5	TRANSVERSE TOP & BOTTOM												

* REQUIRES MECHANICAL COUPLERS (91-#4, 247-#5 & 116-#6)

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NO. 4 BID

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MaryJo.[PLOTTED:

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E-SHEET NAME: S23464

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	BILL OF REINFORCEMENT - L23																	
MARK	TVDE	NO	SIZE	LEN	GTH			4	E	3	(2	[2		E		H
MAIN	TIFE	NO.	JIZE	FT	IN	LOCATION	FT	IN	FT	IN								
							_	_		_		_	_		_			
5S01E	23s	86	#5	4	6	THICKENED SLAB	1	3	1	9	0	11	0	7	0	7 3/4	0	7 3/4
5S02E	23s	86	#5	4	6	THICKENED SLAB	1	3	1	9	0	11	0	7	0	7 3/4	0	7 3/4
5S03E (*)	STR	124	#5	1	7	LONGITUDINAL TOP												
6S01E (*)	STR	180	#6	3	3	LONGITUDINAL BOTTOM												
6S02E	STR	40	#6	7	11	TRANSVERSE BETWEEN STRINGERS												
6S03E (*)	STR	20	#6	46	3	TRANSVERSE TOP & BOTTOM												
6S04E	STR	80	#6	2	4	TRANSVERSE BETWEEN WTs												
6S05E	STR	40	#6	2	5	TRANSVERSE OVER STRINGERS												
6S06E	STR	4	#6	3	11	TRANSVERSE BETWEEN STRINGERS												
6S07E	STR	20	#6	45	5	TRANSVERSE TOP & BOTTOM												

* REQUIRES MECHANICAL COUPLERS (124-#5 & 200-#6)

	BILL OF REINFORCEMENT - L23'																		
		TYDE	NO	017E	LEN	IGTH			Ą		В	(2		D		E		F
		TTPE	NO.	SIZE	FT	IN	LUCATION	FT	IN	FT	IN								
55	601E	23s	86	#5	3	6	THICKENED SLAB	0	10	1	2	0	11	0	7	0	7 3/4	0	7 3/4
55	602E	23s	86	#5	3	4	THICKENED SLAB	0	10	1	0	0	11	0	7	0	7 3/4	0	7 3/4
5S0)3E (*)	STR	124	#5	2	6	LONGITUDINAL TOP												
6S0)1E (*)	STR	180	#6	3	7	LONGITUDINAL BOTTOM												
6S	602E	STR	40	#6	7	11	TRANSVERSE BETWEEN STRINGERS												
6S0)3E (*)	STR	24	#6	46	3	TRANSVERSE TOP & BOTTOM												
6S	604E	STR	40	#6	2	4	TRANSVERSE BETWEEN WTs												
6S	605E	STR	20	#6	2	5	TRANSVERSE OVER STRINGERS												
6S	606E	STR	4	#6	3	11	TRANSVERSE BETWEEN STRINGERS												
6S	607E	STR	24	#6	45	5	TRANSVERSE TOP & BOTTOM												

* REQUIRES MECHANICAL COUPLERS (124-#5 & 204-#6)

BILL OF REINFORCEMENT - L0'																		
MARK	MARK TYPE NO. SIZE LENGTH	NO	CI7E	LENGTH			A		В		С		D		E		F	
WARK		IN	LOCATION		IN	FT	IN	FT	IN	FT	IN	FT	IN	FT	IN			
			_								_	_	_		_		_	_
5S01E	23s	86	#5	3	10	THICKENED SLAB	0	10	1	6	0	11	0	7	0	7 3/4	0	7 3/4
5S02E	23s	74	#5	4	10	THICKENED SLAB	0	10	1	4	1	5	1	3	1	0	1	0
5S03E (*)	STR	138	#5	2	4	LONGITUDINAL TOP												
6S01E (*)	STR	212	#6	3	0	LONGITUDINAL BOTTOM												
6S02E	STR	20	#6	7	11	TRANSVERSE BETWEEN STRINGERS												
6S03E (*)	STR	27	#6	46	3	TRANSVERSE TOP & BOTTOM												
6S04E	STR	40	#6	2	4	TRANSVERSE BETWEEN WTs												
6S05E	STR	20	#6	2	5	TRANSVERSE OVER STRINGERS												
6S06E	STR	2	#6	3	11	TRANSVERSE BETWEEN STRINGERS												
6S07E	STR	27	#6	45	5	TRANSVERSE TOP & BOTTOM												
6S08E	STR	14	#6	12	1	TRANSVERSE BETWEEN GIRDERS												
6S09E	STR	42	#6	3	9	TRANSVERSE BETWEEN WTs												

* REQUIRES MECHANICAL COUPLERS (138-#5 & 239-#6)

N	NOTES							
1.	MECHANICAL COUPLERS SHALL BE EPOXY COATED MECHANICAL BUTT SPLICE TYPE CONFORMING TO SECTION 602.03.06 OF THE SPECIFICATIONS AND APPROVED BY THE ENGINEER.							
	WHERE MECHANICAL COUPLERS ARE REQUIRED, THE REINFORCING BAR LENGTH IS MEASURED TO THE CONSTRUCTION JOINT.							
	BAR LENGTH ADJUSTMENT MAY BE NECESSARY FOR THE TYPE OF MECHANICAL CONNECTOR FURNISHED.							
	PAYMENT FOR MECHANICAL COUPLERS IS CONSIDERED INCIDENTAL TO APPROPRIATE BID ITEM.							
2.	STEEL REINFORCEMENT SHALL BE EPOXY COATED CONFORMING TO SECTION 602 OF SPECIFICATIONS.							
	ESTIMATED WEIGHT OF STEEL REINFORCEMENT - EPOXY COATED							
	PPO 6088 LBS PP23 5570 LBS PP23' 5922 LBS PPO' 6680 LBS							
	PAYMENT FOR STEEL REINFORCEMENT - EPOXY COATED IS CONSIDERED INCIDENTAL TO THE APPROPRIATE BID ITEM.							
3.	CONCRETE USED IN THE SLAB RECONSTRUCTION SHALL BE CLASS "AA" CONFORMING TO SECTION 601 OF THE SPECIFICATIONS.							
	ESTIMATED VOLUME OF CONCRETE - CLASS "AA"							
	PPO 20 CY PP23 21 CY PP23' 20 CY PPO' 27 CY							
	PAYMENT FOR CONCRETE - CLASS "AA" IS CONSIDERED INCIDENTAL TO THE APPROPRIATE BID ITEM.							

		DATE							
	DATE: DEC	BY							
	DESIGNED	BY:M BARON	J STITH						
	DETAILED	BY: MJ DWYER	D BARON						
	Commonwealth of Kentuck DEPARTMENT OF HIGHWA								
	ROUTE		CROSSING OHIO RIVER						
	B	BILL OF RE	OF REINFORCEMENT						
PLAN SET		PREPARED B	Y	SHEET NO.					
	Mi	chael Baker	1650 Lyndon Farm Court Loulsville, KY 40223	S21					
В			Phone: (502) 339-3557 MBAKERINTL COM	DRAWING NO.					
-	INI	ERNATIONAL		28935					

FILE NAME:....\709STR\I-65_SB_JFK\2S7(

USER:morrse DATE PLOTTED:2/14/2017

11 7 113 MODEL NAME, PORTOR

DRAINAGE TROUGH CATCH BASIN AND DOWNSPOUT AT FINGER PLATE EXPANSION JOINTS AT PANEL POINTS L23 AND L23'

SCALE: 1/2"=1'-0"

NOTES: PANEL POINT L23' SHOWN. PANEL POINT L23 OPPOSITE HAND. INTERIOR CATCH BASIN SHOWN. DETAILS FOR EXTERIOR CATCH BASIN SIMILAR.

© I-¾"DIA A325 BOLT W∕ WASHER AND HEAVY HEX NUT. FIELD DRILL 15/16 " DIA. HOLES THROUGH STRINGER WEB

SB

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MODEL

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9' CH		
-0-0		
2S7046 AND 2S7047. AT THE FINGER ITATIONS ONLY.	WALSH AS-BUILT	JOHN E. JOHN E. SB920 CENSED DRAL ST
ON ONLY	REV. 00 RFC SUBMITTAL REVISION NO. SUBMITTAL NAME SECTION 2 - ORB DOWN I-65 SB - JFK BRIDGE SLAB DRAINAGE - SHE PREPARED BY	NTOWN REHAB ET 4 Drawing No.

ITEM NO.

COUNTY OF

SHEET NO.

2S7047A

